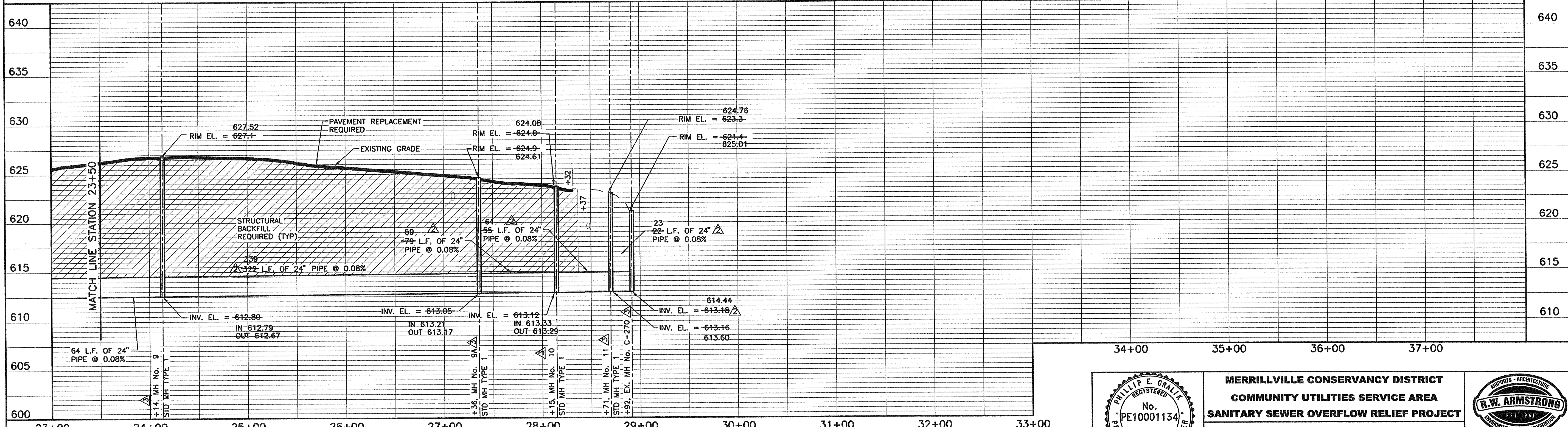


**PLAN**  
SCALE: 1"=50'

**NOTES**

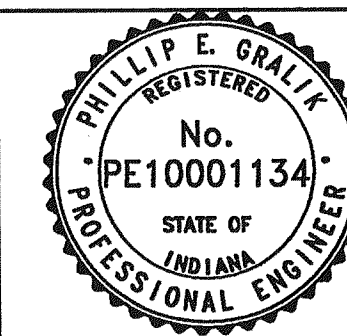
1. SEE ADDITIONAL PLAN SHEETS 1 AND 2 FOR ADDITIONAL WORK ITEMS ADDED UNDER CHANGE ORDER NO. 1.
2. LINE "C" AND "D" HAVE BEEN ADDED TO THE PROJECT UNDER CHANGE ORDER NO. 1, SEE ADDITIONAL PLAN SHEETS 1 AND 2.
3. ALL STATIONING ON THIS SHEET HAS BEEN REVISED DUE TO THE ALIGNMENT CHANGES BETWEEN MANHOLES 5 AND 7 AND SHIFTING THE SEWER TO THE EAST HALF OF STREET.

**RECORD DRAWING**  
DRAWN BY: RWP  
DATE: 8-24-05



**PROFILE**  
SCALE: HORIZ. : 1"=50'  
VERT. : 1"=5'

NO.	DATE	REVISIONS	MOB	PEG
4	5/13/04	REVISED STATIONING AFTER MOVING MH 4-7	GLM	PEG
3	2/13/04	ADDED CHANGE ORDER NO. 1 ITEMS (WWTP DEMO, LINE "C" AND "D")	GLM	PEG
1	10/6/03	DELETED DEMOLITION OF EXISTING WWTP (ADDENDUM #2)	GLM	PEG
			BY	APPR.

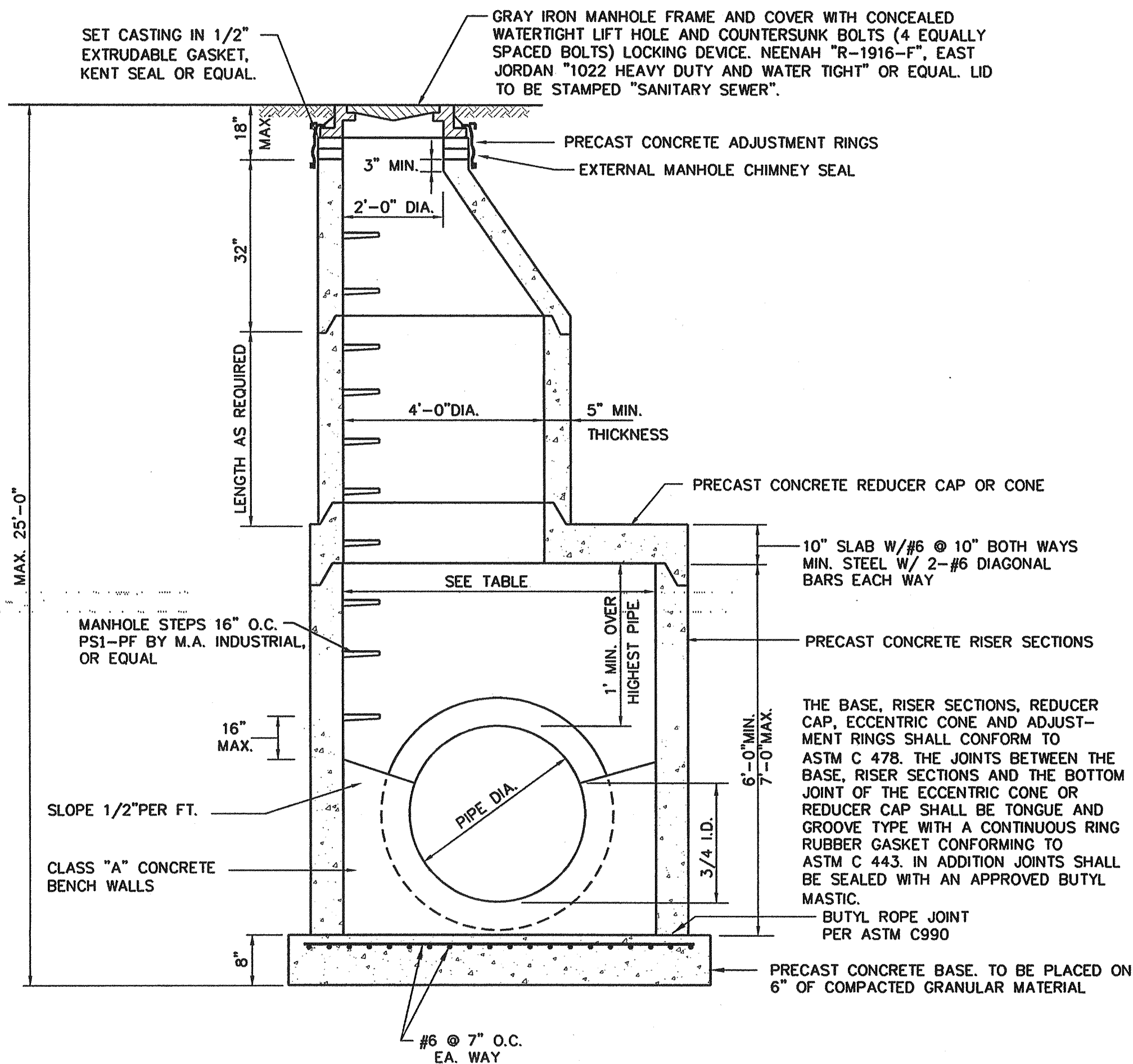


**MERRILLVILLE CONSERVANCY DISTRICT**  
**COMMUNITY UTILITIES SERVICE AREA**  
**SANITARY SEWER OVERFLOW RELIEF PROJECT**  
**PLAN AND PROFILE**  
**LINE "B" STATION 23+50 TO 28+90**

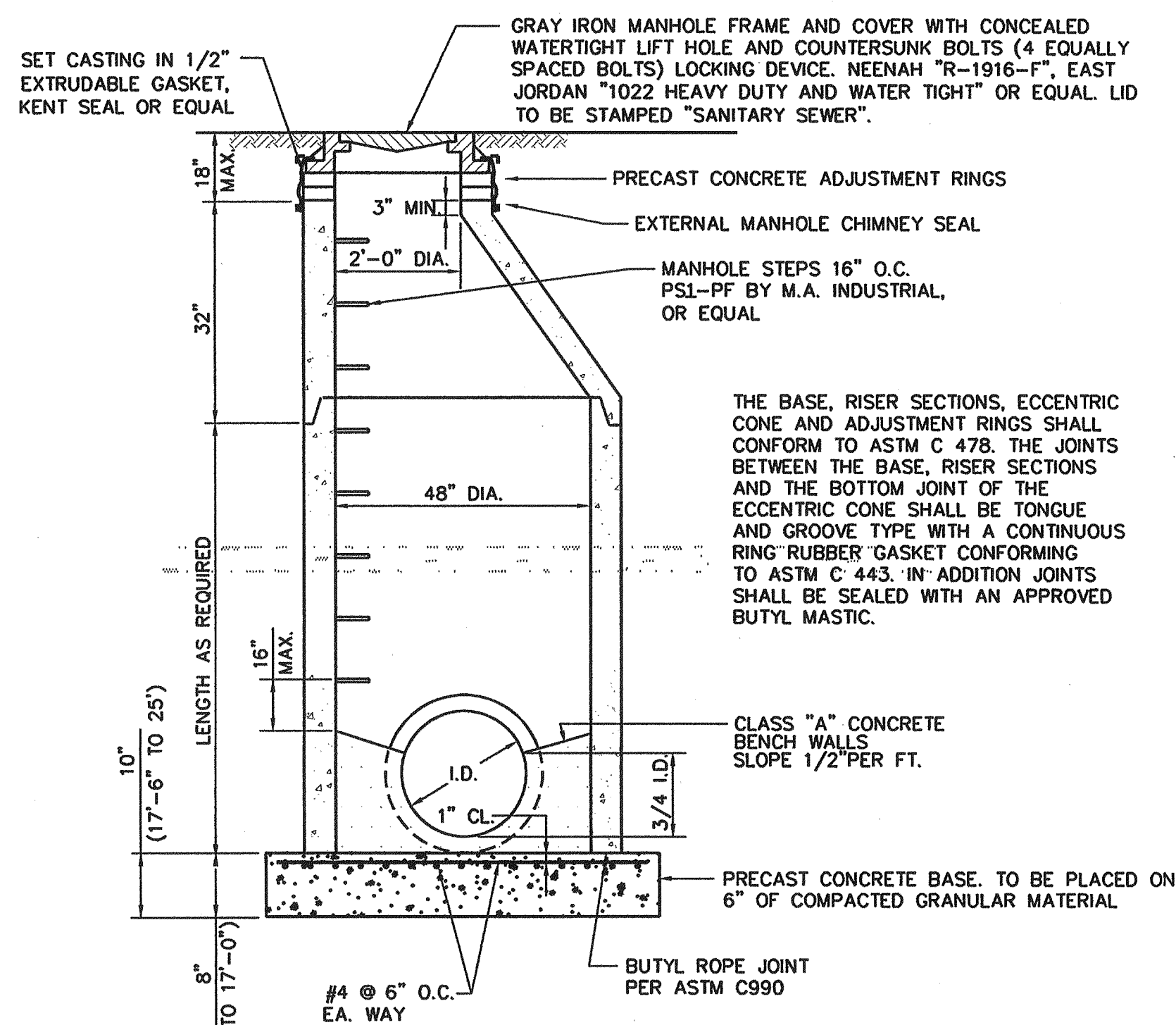
JOB NO.	20016400.10	DRAWN	RWP	SCALE:	AS NOTED
DATE	4/03	DESIGNED	AJS	APPROVED	RDK



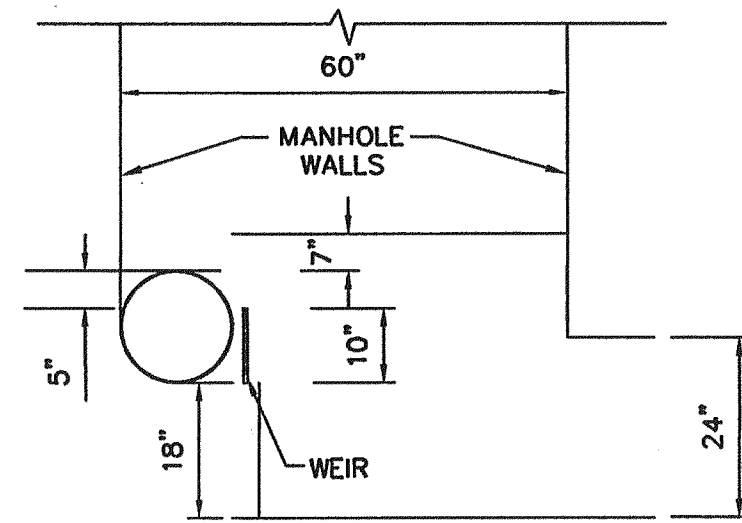




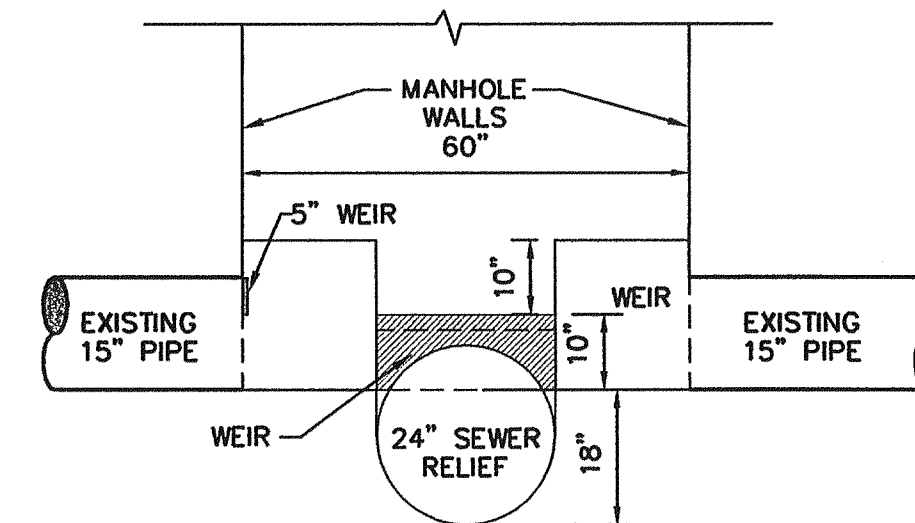
**STANDARD PRECAST CONCRETE MANHOLE - TYPE 2**  
SEE TABLE FOR MAXIMUM INSIDE DIAMETER FOR CONNECTING SEWER  
NOT TO SCALE



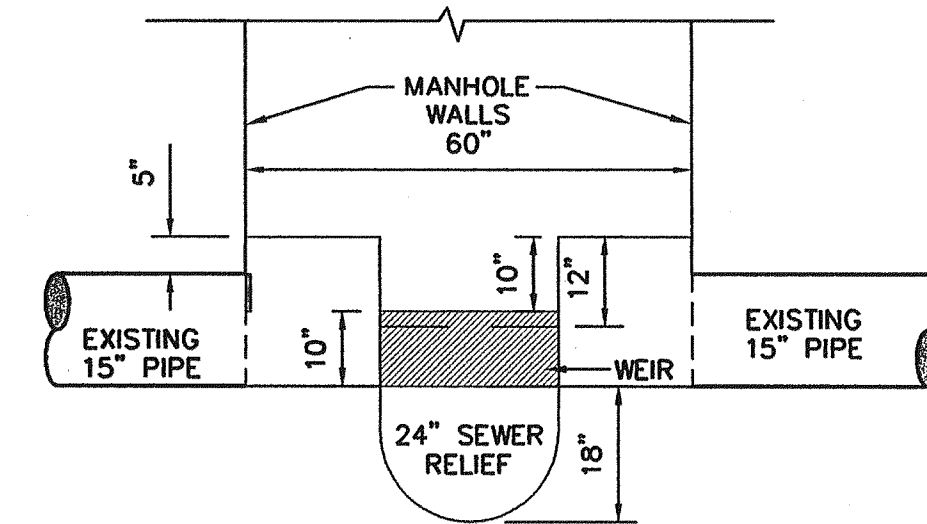
**STANDARD PRECAST CONCRETE MANHOLE - TYPE 1**  
NOT TO SCALE



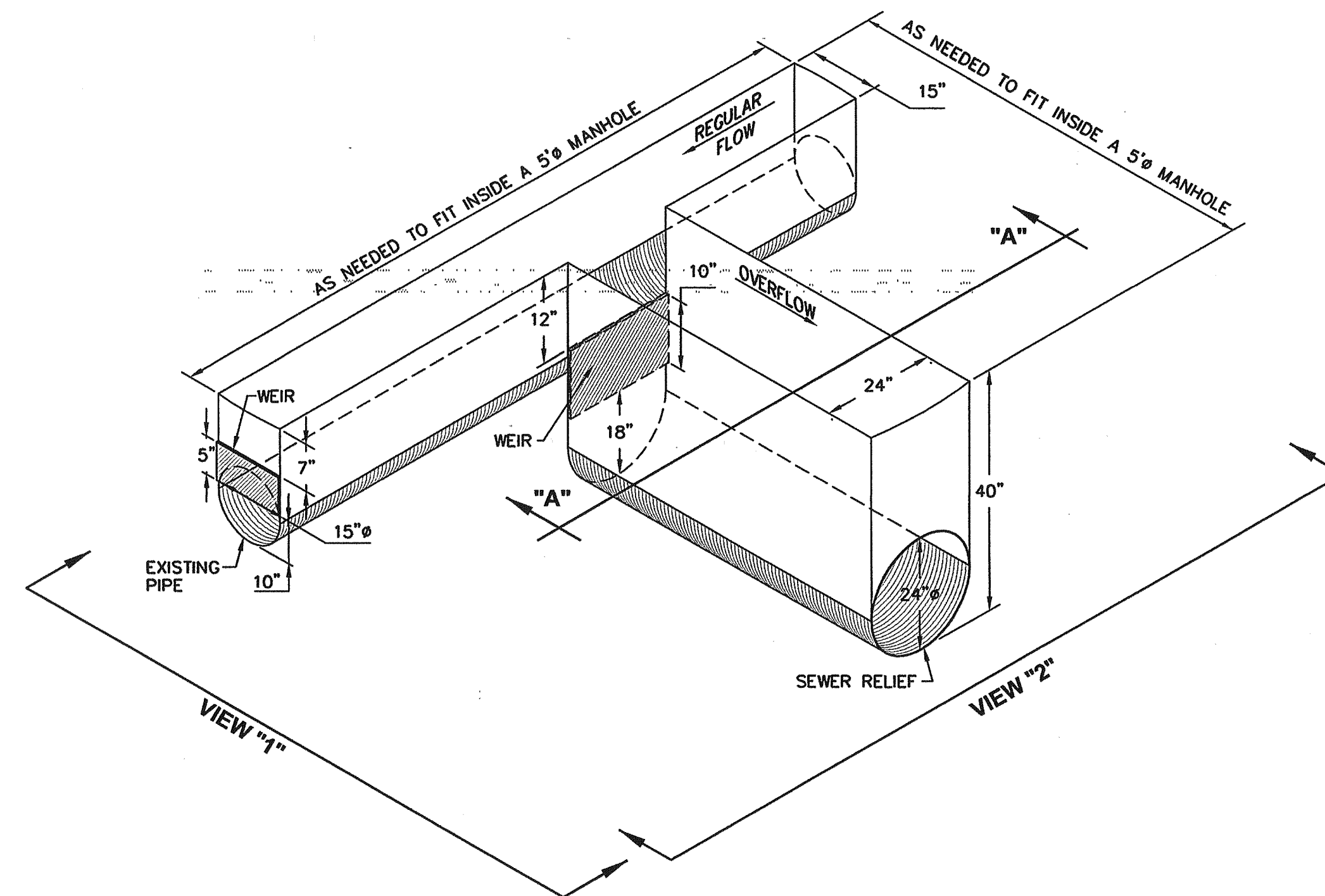
**VIEW '1'**  
NOT TO SCALE



**VIEW '2'**  
NOT TO SCALE



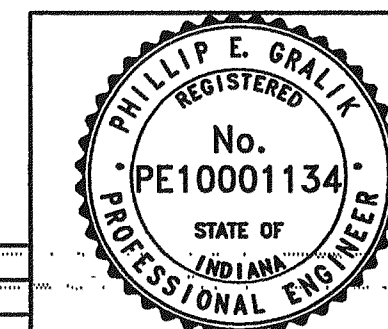
**SECTION 'A-A'**  
NOT TO SCALE



**OVERFLOW STRUCTURE TROUGH DETAIL**  
NOT TO SCALE

MINIMUM MANHOLE DIAMETER FOR VARIOUS SIZE SEWERS				
MIN. INSIDE MANHOLE DIAMETER, INCHES	SEWER PIPE DEFLECTION THROUGH MANHOLE, DEGREES	TYPE OF SEWER PIPE & SIZE		
		PVC	CLAY	DI/ FIBER
48	0 - 45	27"	21"	24"
48	46 - 90	21"	18"	20"
60	0 - 45	36"	30"	36"
60	46 - 90	27"	21"	24"
72	0 - 45	42"	36"	42"
72	46 - 90	30"	24"	30"
84	0 - 45	48"	42"	48"
84	46 - 90	36"	30"	36"
96	46 - 90	42"	36"	42"
108	46 - 90	48"	42"	48"

NO.	DATE	REVISIONS	BY	APPR.
4				
3				
2				
1				



MERRILLVILLE CONSERVANCY DISTRICT COMMUNITY UTILITIES SERVICE AREA SANITARY SEWER OVERFLOW RELIEF PROJECT				
MISCELLANEOUS DETAILS				
JOB NO.	20016400.10	DRAWN	RWP	SCALE: AS NOTED
DATE	4/03	DESIGNED	AJS	APPROVED RDK

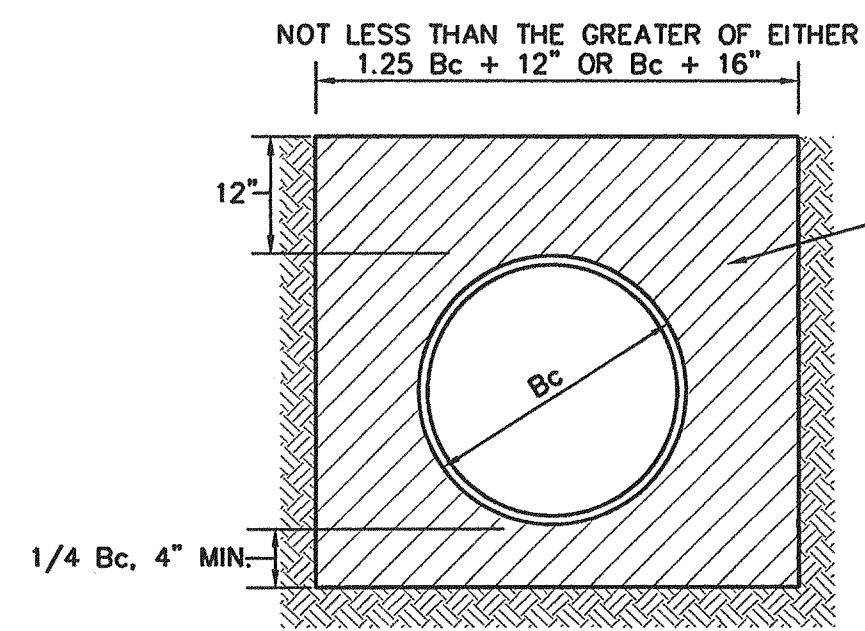
**RECORD DRAWING**  
DRAWN BY: RWP  
DATE: 8-24-05

**R.W. ARMSTRONG**  
EST. 1961  
ARCHITECTURAL - TRANSPORTATION

2801 S. PENNSYLVANIA STREET  
INDIANAPOLIS, INDIANA 46225 (317) 788-0461

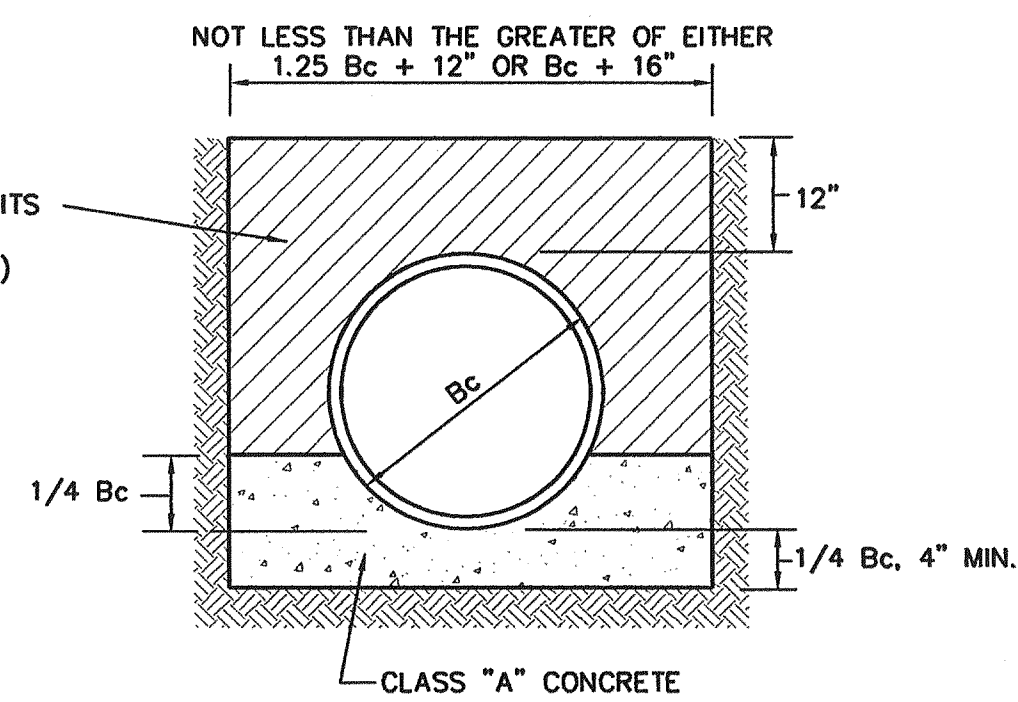
SHEET 7 OF 8





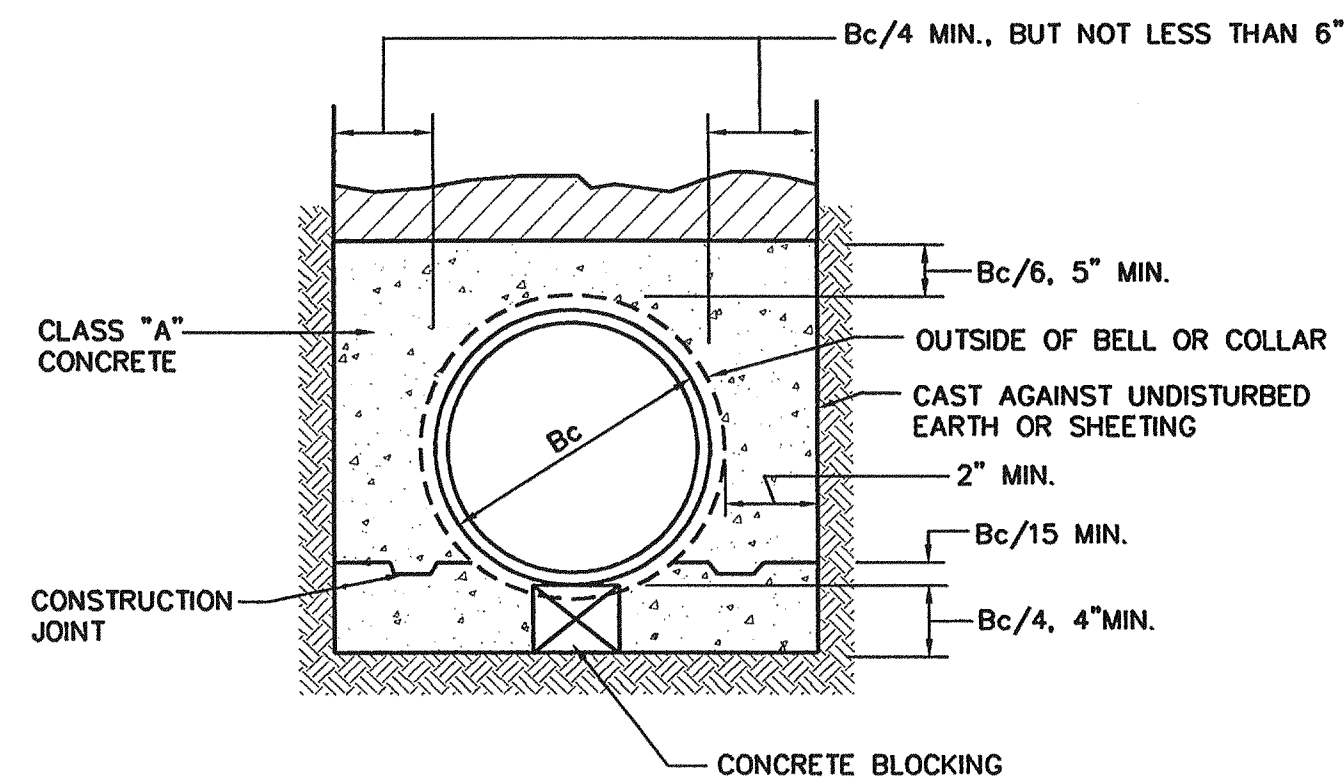
NOTE: IN ROCK TRENCH, EXCAVATE AT LEAST 6" BELOW THE BELL OF THE PIPE.

### STANDARD BEDDING

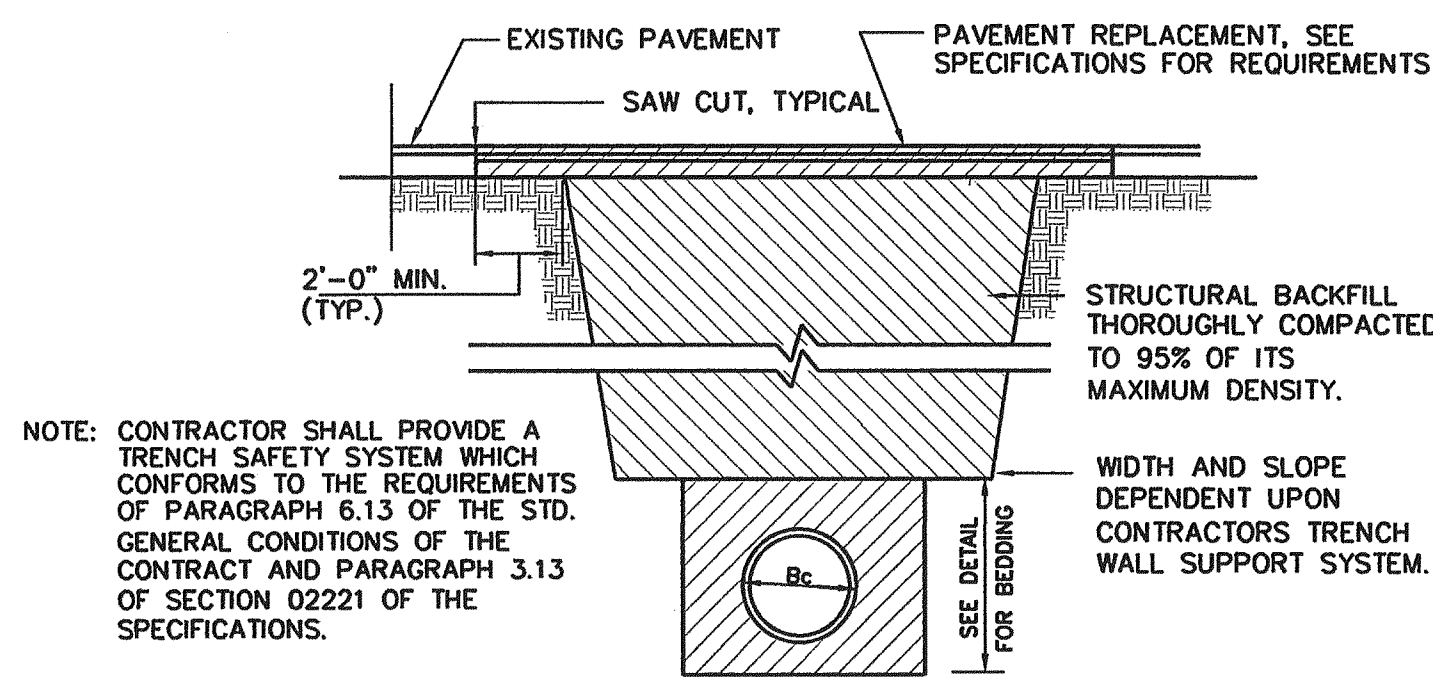


### CONCRETE CRADLE

### BEDDING DETAILS



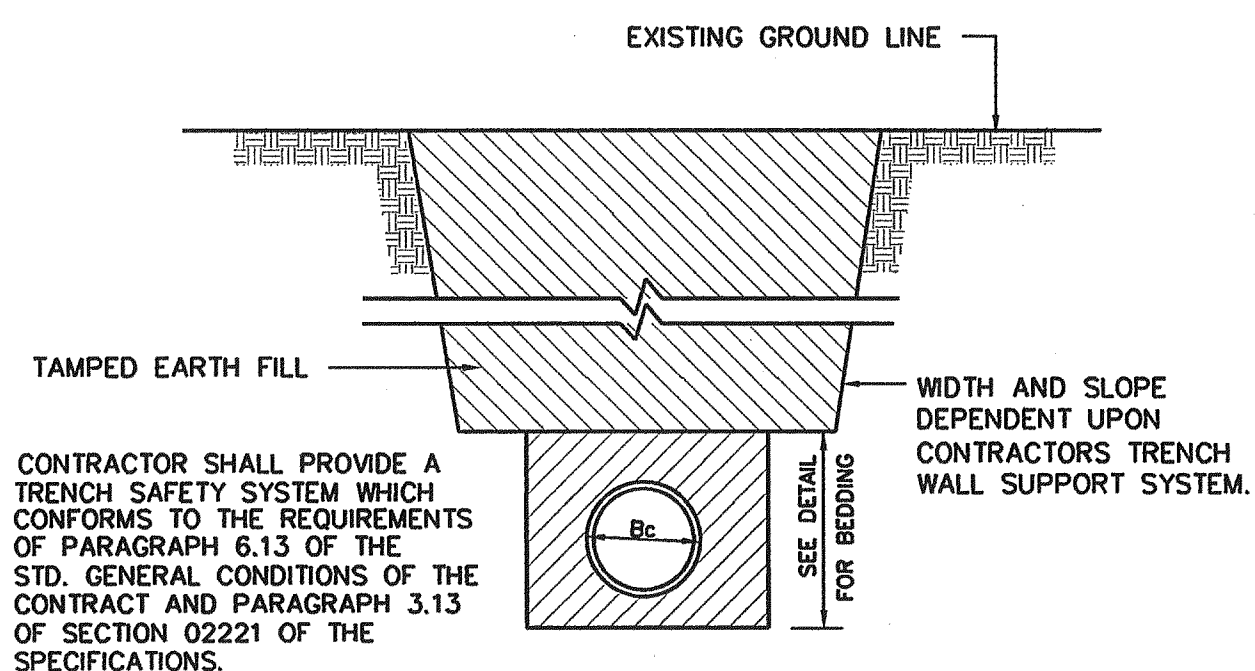
### CONCRETE ENCASEMENT



NOTE: CONTRACTOR SHALL PROVIDE A TRENCH SAFETY SYSTEM WHICH CONFORMS TO THE REQUIREMENTS OF PARAGRAPH 6.13 OF THE STD. GENERAL CONDITIONS OF THE CONTRACT AND PARAGRAPH 3.13 OF SECTION 02221 OF THE SPECIFICATIONS.

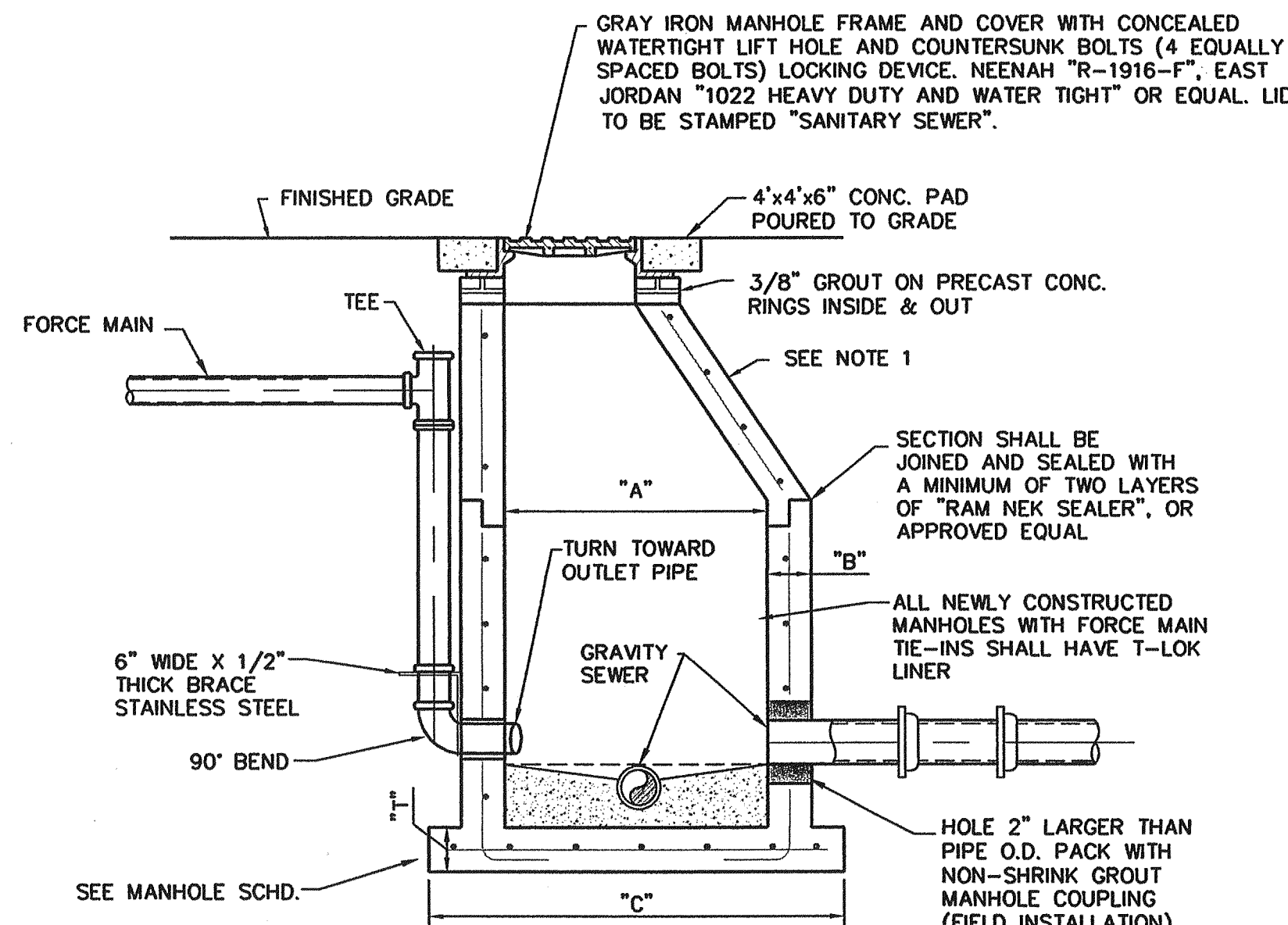
### TRENCH AND BACKFILL UNDER PAVEMENT AND OTHER LOCATIONS DETAIL

\*AS SPECIFIED IN SECTION 02221 OF THE SPECIFICATIONS



NOTE: CONTRACTOR SHALL PROVIDE A TRENCH SAFETY SYSTEM WHICH CONFORMS TO THE REQUIREMENTS OF PARAGRAPH 6.13 OF THE STD. GENERAL CONDITIONS OF THE CONTRACT AND PARAGRAPH 3.13 OF SECTION 02221 OF THE SPECIFICATIONS.

### TRENCH AND BACKFILL DETAIL



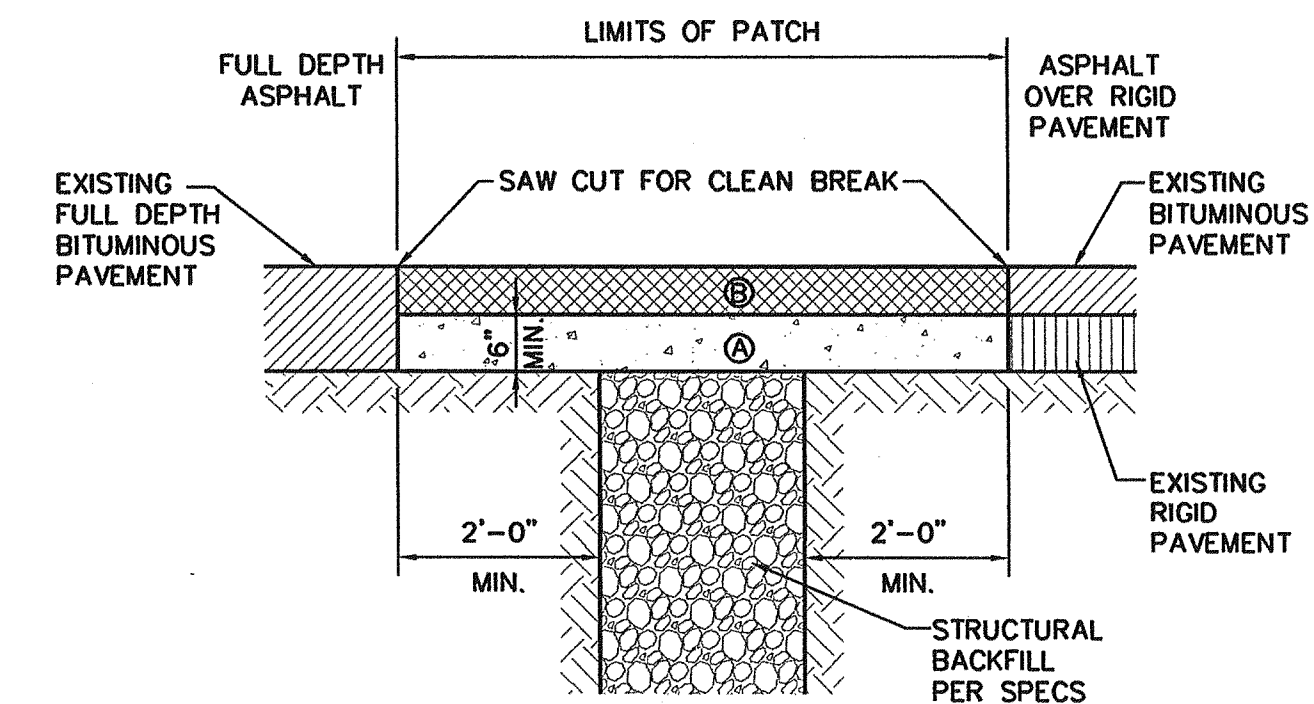
PIPE DIA.	"A"	"B"	"C"	BOT. SLAB "T"
8"-24"	4'-0"	"8"	70"	8"
30"-36"	5'-0"	"8"	84"	10"
42"-48"	6'-0"	"8"	96"	12"

TYPICAL MANHOLE DIMENSIONS

#### NOTES:

- ALL CONCRETE MANHOLES TO BE 4000 P.S.I. TO MEET OR EXCEED ASTM C478 ALL CEMENT TO BE TYPE II ACID RESISTANT. REINFORCING AREA OF 0.20 SQ. IN/FT FOR WALL SECTION MIN. TO MEET OR EXCEED ASTM A 185.
- A MAXIMUM OF 2 LAYERS OF PRECAST CONCRETE RINGS, IF REQUIRED.
- FORCE MAIN TIE-IN SHALL BE TO MANHOLES WITH A MINIMUM DEPTH OF 4'-6"

### FORCE MAIN TIE-IN AT PRECAST MANHOLE

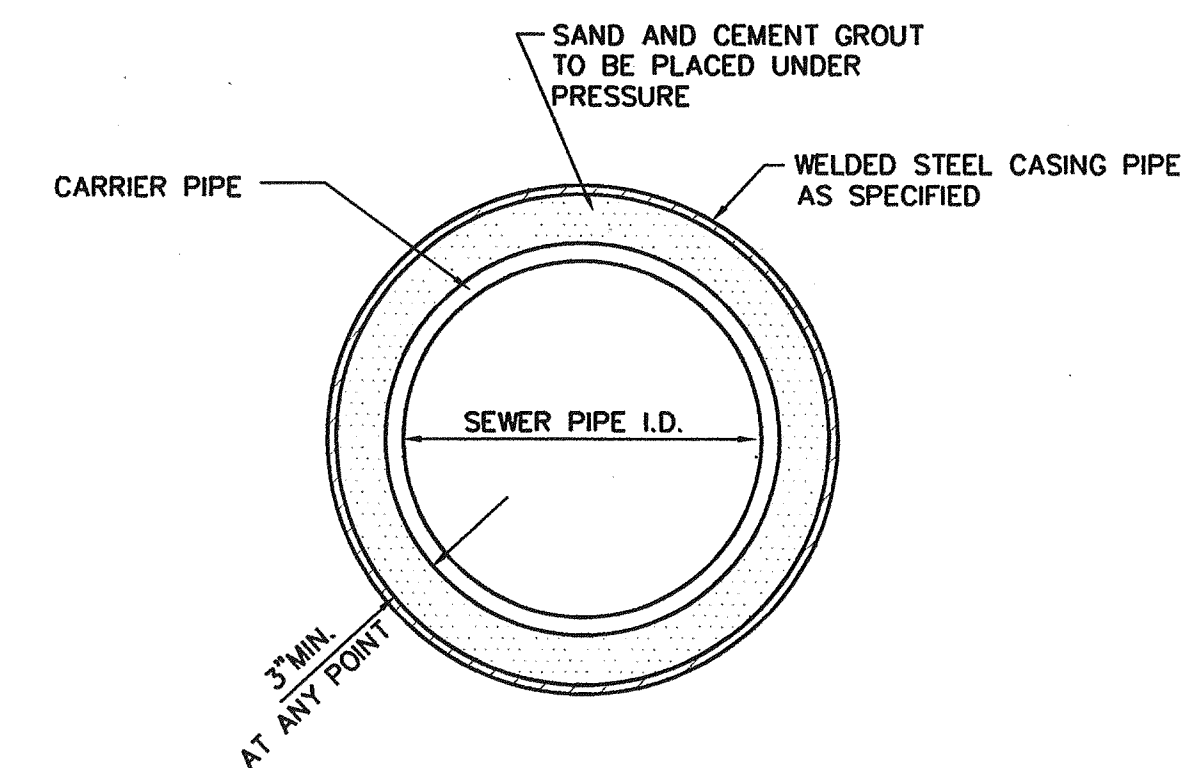


- PLAIN CONCRETE IS TO BE FINISHED FLUSH TO THE EXISTING RIGID PAVEMENT OR 2" BELOW EXISTING ASPHALT SURFACE.
- HOT ASPHALTIC MATERIAL IS TO BE FINISHED FLUSH TO THE EXISTING BITUMINOUS MATERIAL AND IS TO BE COMPACTED TO 95%.

#### NOTES

- NEW SURFACE IS TO BE SLOPED AT THE SAME RATE AS THE EXISTING SURFACE.
- EXISTING PAVEMENT IS TO BE SAW-CUT FOR A CLEAN BREAK.
- COMPACTED AGGREGATE IS TO BE LAID AT 6" LIFTS AND IS TO BE COMPACTED TO A MINIMUM OF 100% OF MAXIMUM DRY DENSITY.
- TRENCH SPOIL IS TO BE REMOVED FROM THE WORK SITE.
- BITUMINOUS TACK APPLIED AS PER CURRENT ISSUE OF "INDIANA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS."
- ALL JOINTS SHALL BE SAW-CUT TO A DEPTH OF 4" MINIMUM. BEFORE FINAL REMOVAL IS PERFORMED, ALL JOINTS BETWEEN EXISTING PAVEMENT AND NEW CONCRETE SHALL BE SMOOTH AND STRAIGHT. (CONCRETE AND ASPHALT PAVEMENT)
- HI-EARLY CLASS A (SLAG) CONCRETE WITH 6%-8% AIR ENTRAINMENT WILL BE ALLOWED FOR SPECIAL APPLICATIONS, UPON APPROVAL OF PERMITTING AGENCY.
- BLACK CONCRETE FINISH SHALL BE USED IN LOCATIONS AS DIRECTED BY THE PERMITTING AGENCY.
- PAVEMENT PATCH SHALL BE SMOOTH AND LEVEL. NO MORE THAN 1/4" VARIATION FROM A STRAIGHT EDGE 5'-0" IN LENGTH SHALL BE ALLOWED. SMOOTH RIDE ABILITY MUST BE MAINTAINED.

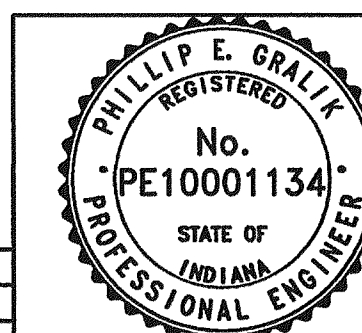
### ROAD CUT PATCHING DETAIL



#### CROSS-SECTION

### PIPE IN CASING

4					
3					
2					
1					
NO.	DATE	REVISIONS	BY	APPR.	



MERRILLVILLE CONSERVANCY DISTRICT COMMUNITY UTILITIES SERVICE AREA SANITARY SEWER OVERFLOW RELIEF PROJECT			
MISCELLANEOUS DETAILS			
JOB NO.	20016400.10	DRAWN	RWP
DATE	4/03	DESIGNED	AJS
SCALE:	AS NOTED	APPROVED	RDK

RECORD DRAWING
DRAWN BY: RWP
DATE: 8-24-05
2601 S. PENNSYLVANIA STREET INDIANAPOLIS, INDIANA 46226 (317) 786-0461
SHEET 8 OF 8



## **APPENDIX L**

### **MCD'S WESTSIDE INTERCEPTOR SEWER PROJECT CONTRACT PLANS**



# MERRILLVILLE CONSERVANCY DISTRICT

## LAKE COUNTY, INDIANA

### WESTSIDE INTERCEPTOR SEWER PROJECT

SHEET INDEX	
SHEET NO.	DESCRIPTION
2	TITLE SHEET
3	PLAN SHEET INDEX, GENERAL NOTES & SYMBOLS
4	PLAN & PROFILE - LINE "A", STA. 14+05 TO STA. 25+25
5	PLAN & PROFILE - LINE "A", STA. 25+25 TO STA. 34+50
6	PLAN & PROFILE - LINE "A", STA. 34+50 TO STA. 49+50
7	PLAN & PROFILE - LINE "A", STA. 49+50 TO STA. 62+50
8	PLAN & PROFILE - LINE "A", STA. 62+50 TO STA. 77+50
9	PLAN & PROFILE - LINE "A", STA. 77+50 TO STA. 92+50
10	PLAN & PROFILE - LINE "A", STA. 92+50 TO STA. 107+50
11	PLAN & PROFILE - LINE "A", STA. 107+50 TO STA. 121+50
12	PLAN & PROFILE - LINE "A", STA. 121+50 TO STA. 136+50
13	PLAN & PROFILE - LINE "A", STA. 136+50 TO STA. 143+77 & LINE "B"
14	MISCELLANEOUS DETAILS

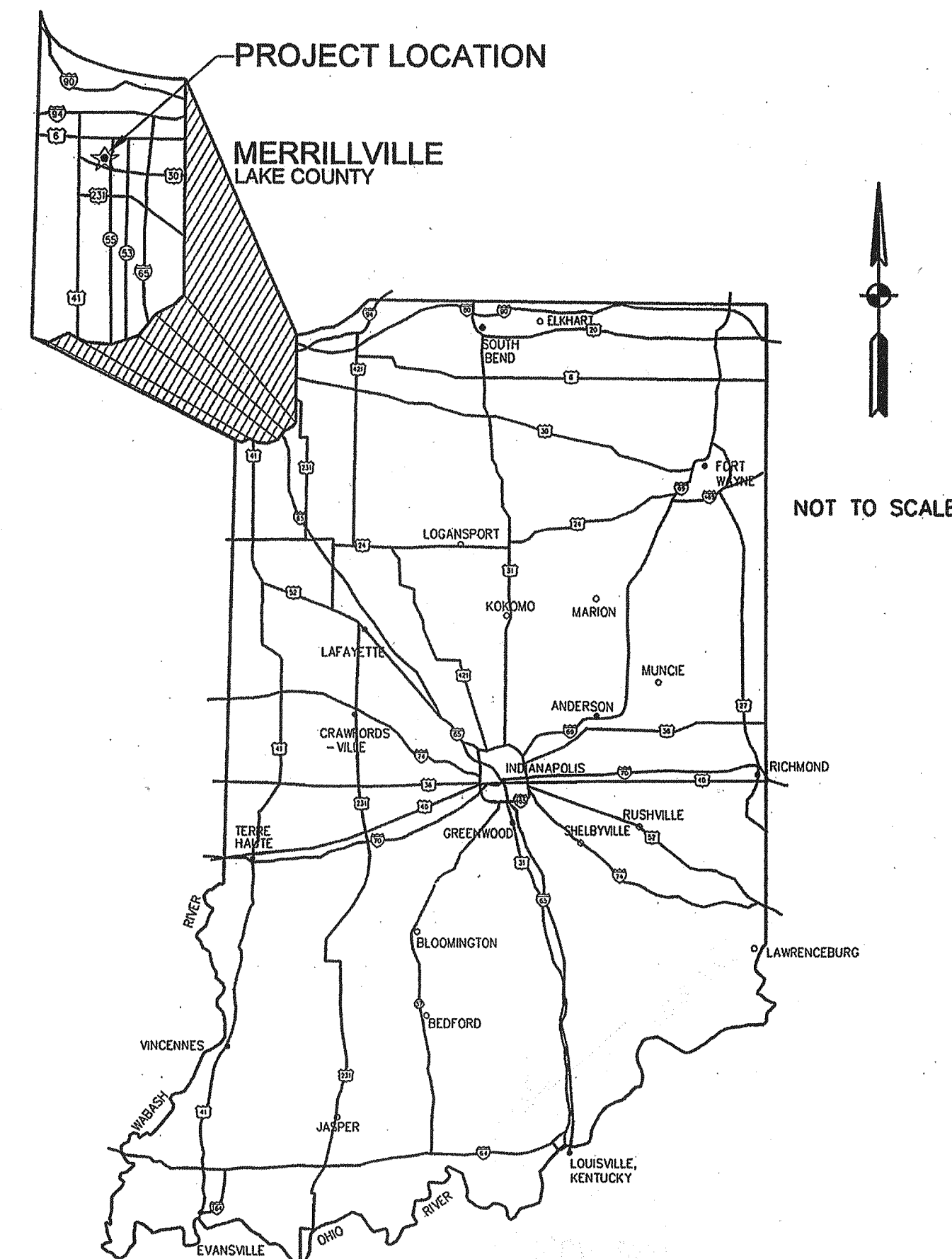
EROSION CONTROL LEGEND (TYPICAL)

SILT FENCE ————

EROSION BLANKETS ————

PERMANENT SEED ————

WETLAND SEED ————



LOCATION MAP



UNION STATION 300 S. MERIDIAN STREET  
INDIANAPOLIS, INDIANA 46225  
(317) 786-0461

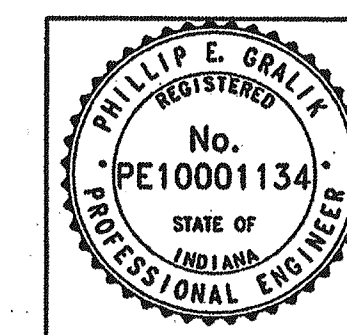
8300 BROADWAY, SUITE E-1  
MERRILLVILLE, INDIANA 46410  
(219) 738-2258

APPROVED BY : MERRILLVILLE  
CONSERVANCY DISTRICT  
BOARD OF DIRECTORS

<i>Paul L. Volk</i>	12-9-03
PAUL L. VOLK	CHAIRMAN OF THE BOARD
<i>Milan Dakich</i>	12-9-03
MILAN DAKICH	VICE-CHAIRMAN
<i>Joseph T. Sanok</i>	12-9-03
JOSEPH T. SANOK	SECRETARY/TREASURER
ABSENT	
THOMAS P. KEILMAN	BOARD MEMBER
<i>Edward J. Westbury</i>	12-09-03
EDWARD J. WESTBURY	BOARD MEMBER
<i>Christine V. Savarese</i>	12-09-03
CHRISTINE V. SAVARESE	DISTRICT MANAGER

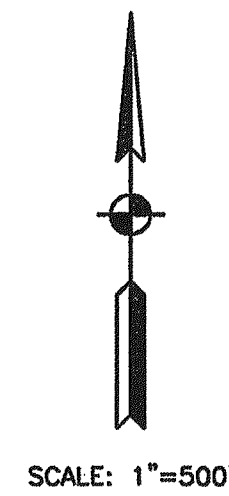
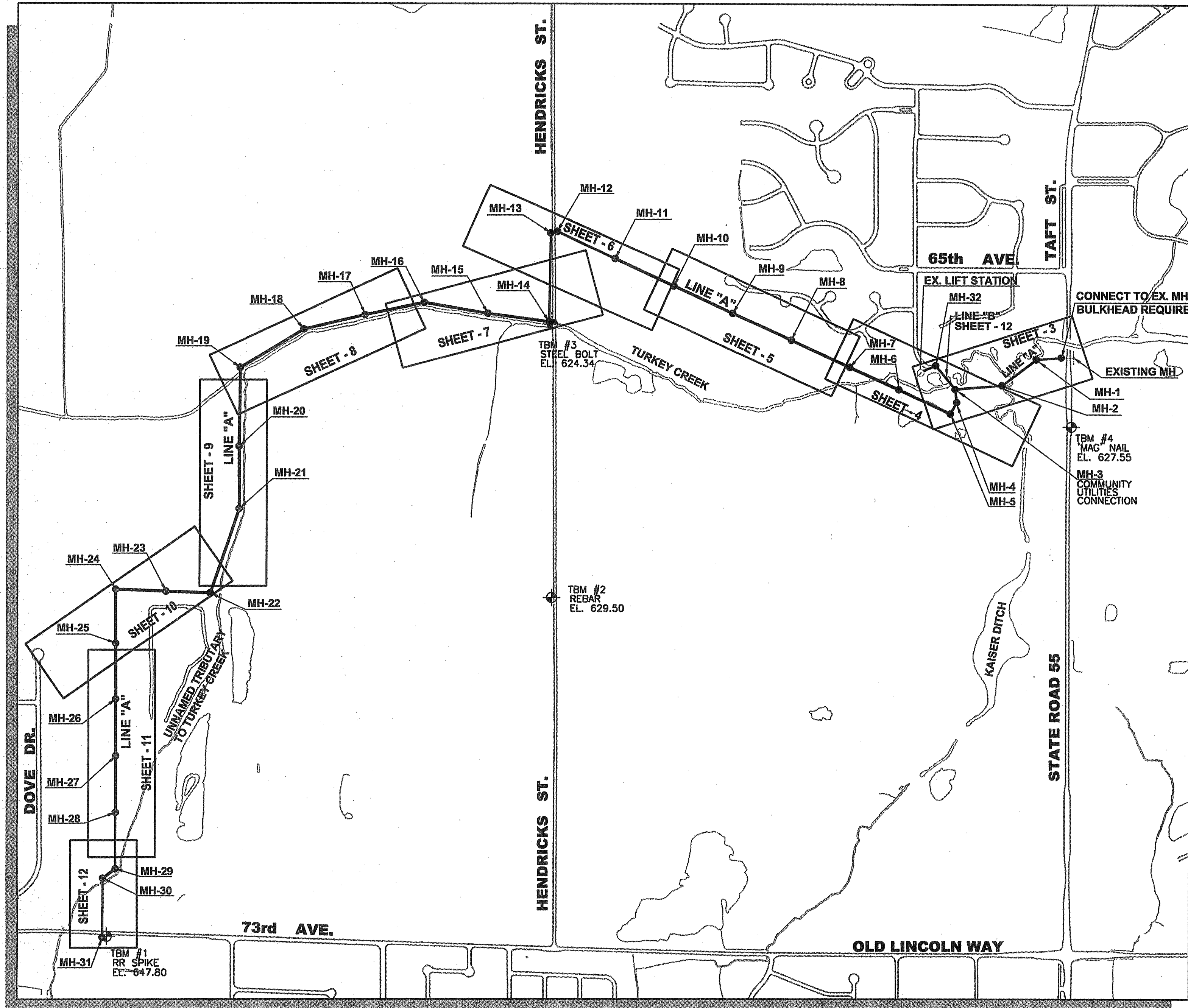
CERTIFIED BY:

*Phillip E. Gralik* 12/05/03  
PHILLIP E. GRALIK, P.E. 10001134 DATE



Set No. 20





GENERAL NOTES

1. THE CONTRACTOR SHALL REVIEW AND TAKE NOTE OF THE REQUIREMENTS INCLUDED IN SPECIFICATIONS SECTION 1, PARAGRAPH 6 "STRUCTURES ENCOUNTERED". UTILITY BUILDING SERVICE LINES ARE NOT INDICATED ON THE PLANS. ALL SERVICE LINES CUT OR DAMAGED BY THE CONTRACTOR SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE TO A CONDITION EQUAL TO OR BETTER THAN THE CONDITION OF THE ORIGINAL LINES, OR BE REPLACED WITH A LINE OF THE SAME MATERIAL AND SIZE.
2. THE CONTRACTOR SHALL INSTALL A SECURE WATERTIGHT BULKHEAD OR PLUG IN THE SANITARY SEWER TO PREVENT FLOODING OF THE FACILITIES OR ADJACENT CONNECTING FACILITIES. THE BULKHEAD OR PLUG SHALL BE INSTALLED AT THE POINT OF CONNECTION TO THE EXISTING MANHOLE AT TAFT STREET. THE CONTRACTOR SHALL ONLY REMOVE THE BULKHEAD OR PLUG AFTER SATISFACTORY COMPLETION OF TESTING AND ANY REQUIRED CORRECTIVE WORK IN THE VARIOUS PORTIONS OF THE WORK AND UPON APPROVAL OF THE OWNER.
3. THE CONTRACTOR MAY CLOSE LOCAL ROADWAYS ONLY WITH THE APPROVAL OF THE APPROPRIATE HIGHWAY/STREET AUTHORITY AND AFTER DEVELOPING AND IMPLEMENTING A PLAN TO MAINTAIN LOCAL TRAFFIC AND EMERGENCY VEHICLE TRAFFIC AT ALL TIMES. THE CONTRACTOR SHALL PROVIDE LOCAL DETOUR AND OTHER SIGNAGE AND BARRICADES IN ACCORDANCE WITH INDOT SPECIFICATIONS. TRAFFIC MAINTENANCE AND SIGNAGE ALONG STATE ROAD 55 SHALL BE PER INDOT SPECIFICATIONS.
4. PRIOR TO BEGINNING ANY CONSTRUCTION WORK THE CONTRACTOR SHALL VERIFY ALL BENCH MARK ELEVATIONS AS WELL AS ELEVATIONS OF EXISTING SANITARY SEWERS AT THEIR POINT OF CONNECTION WITH THIS PROJECT. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY DISCREPANCIES FOUND BETWEEN HIS MEASURED ELEVATIONS AND THOSE SHOWN ON THE PLANS.
5. THE CONTRACTOR SHALL ROTATE THE ECCENTRIC CONE OF MANHOLES OR TAKE OTHER POSSIBLE ACTION TO LOCATE MANHOLE COVERS OUTSIDE OF THE WHEEL PATH OF VEHICLES WHERE MANHOLES ARE INSTALLED UNDER PAVEMENT.
6. DEMOLITION OF EXISTING COMMUNITY UTILITIES LIFT STATION SHALL NOT BEGIN UNTIL THE ENTIRE SEWER SYSTEM IN THIS PROJECT HAS BEEN INSTALLED, AND ALL TESTING COMPLETED AND ACCEPTED BY THE OWNER AND ENGINEER.
7. ALL SALVAGEABLE EQUIPMENT SHALL REMAIN THE PROPERTY OF THE OWNER AND SHALL BE REMOVED AND STORED AT A LOCATION DIRECTED BY THE OWNER.

SYMBOLS

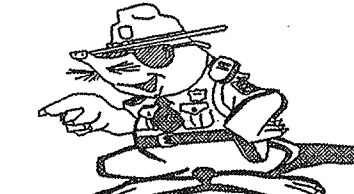
	EXISTING POWER POLE
	EXISTING UTILITY POLE
	EXISTING MAIL BOX
	EXISTING POST
	EXISTING SIGN
	EXISTING TELEPHONE PEDESTAL
	EXISTING SANITARY SEWER MANHOLE
	EXISTING BEEHIVE INLET
	EXISTING STORM CURB INLET
	EXISTING GAS VALVE
	EXISTING BUTTERFLY VALVE
	EXISTING FIRE HYDRANT W/ AUXILIARY VALVE
	EXISTING WATER METER
	EXISTING OVERHEAD ELECTRIC
	EXISTING WATER MAIN (SIZE AND TYPE NOTED)
	EXISTING GAS MAIN
	EXISTING BURIED TELEPHONE
	EXISTING CULVERT PIPE
	EXISTING EASEMENT
	EXISTING FENCE
	EXISTING TREE
	EXISTING HEDGE
	EXISTING TREE LINE
	EXISTING BUSH
	APPROXIMATE PROPERTY LINE
	APPROXIMATE RIGHT-OF-WAY
	EXISTING 1' CONTOUR LINE
	EXISTING 5' CONTOUR LINE
	NEW SANITARY SEWER AND MANHOLE

ABBREVIATIONS

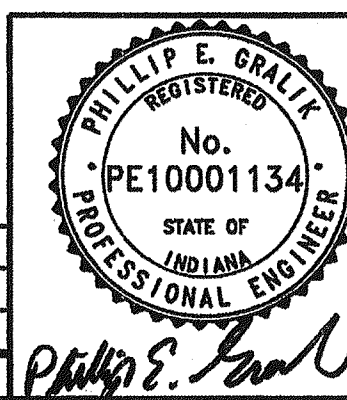
CL	CENTER LINE
CMP	CORRUGATED METAL PIPE
ESMT	EASEMENT
E	EAST
EL	ELEVATION
HORIZ.	HORIZONTAL
INV.	INVERT
INV. EL.	INVERT ELEVATION
L.F.	LINEAR FEET
MH	MANHOLE
N	NORTH
P.E.	PERMANENT EASEMENT
PVC	POLYVINYL CHLORIDE PIPE
RCP	REINFORCED CONCRETE PIPE
R/W	RIGHT OF WAY
S	SOUTH
STA.	STATION
T.E.	TEMPORARY EASEMENT
TYP.	TYPICAL
W	WEST

BENCHMARKS		
TBM ID #	DESCRIPTION	ELEVATION
1	RAILROAD SPIKE SET IN SOUTH FACE OF WOOD UTILITY POLE 808/758 N 2272243.9198 E 2868143.1154	647.80
2	HUB SET SOUTH SIDE OF DRIVE TO VFW, 3' EAST OF RECTANGULAR STEEL POST 10' NORTH OF STEEL FENCE POST N 2274866.2353 E 2871583.5203	629.50
3	2" Ø STEEL BOLT WITH PUNCH MARK IN CENTER NORTHWEST END OF BRIDGE ON WOOD ABUTMENT N 2276990.6903 E 2871584.9066	624.34
4	"MAG" NAIL SET IN PAINT STRIPING OF TURNING LANE, 9.7' WEST OF STRAIN POLE NO. 863/844, 15.85' NORTHWEST OF FENCE CORNER N 2276190.7795 E 2875599.3948	627.55

HOLEY MOLEY SAYS  
**"DON'T  
DIG  
BLIND"**



**"IT'S THE LAW"**  
CALL 2 WORKING DAYS BEFORE YOU DIG  
**1-800-382-5544**  
CALL TOLL FREE  
PER INDIANA STATE LAW 6-59-1991,  
IT IS AGAINST THE LAW TO EXCAVATE  
WITHOUT NOTIFYING THE UNDERGROUND  
LOCATION SERVICE TWO (2) WORKING  
DAYS BEFORE COMMENCING WORK.



MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT

PLAN SHEET INDEX  
GENERAL NOTES AND SYMBOLS

JOB NO. 20024530.1	DRAWN	RWP/MOB	SCALE:	AS NOTED
DATE 8/03	DESIGNED	AJS/GLM	APPROVED	AJS



2801 S. PENNSYLVANIA STREET  
INDIANAPOLIS, INDIANA 46225 (317) 786-6461

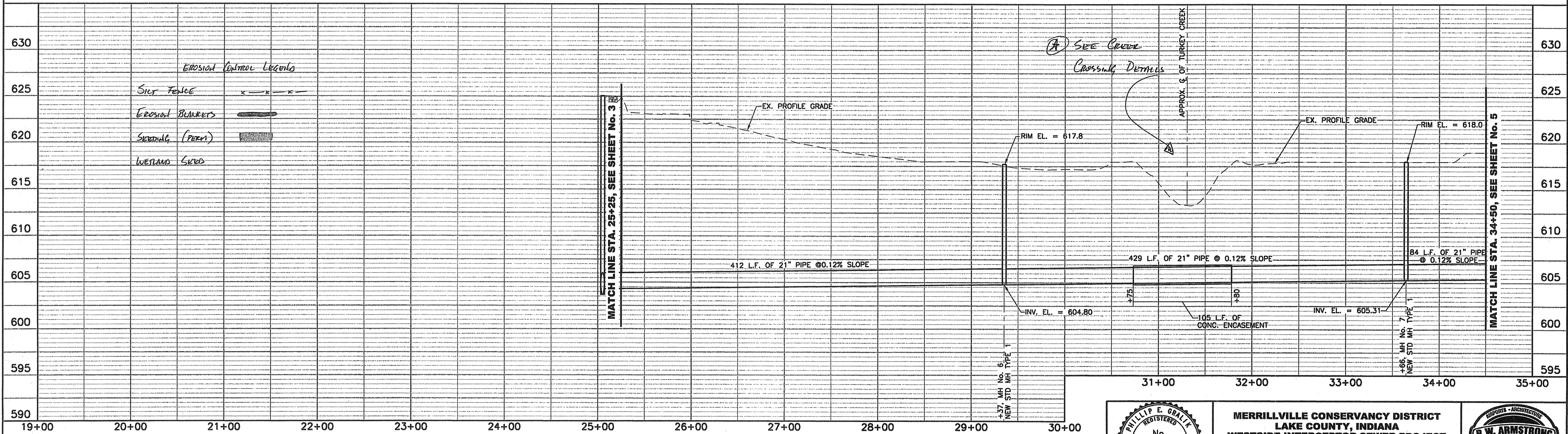






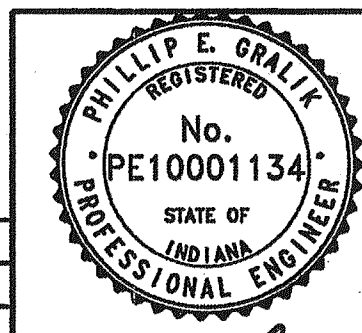


PLAN



PROFILE

3					
2					
1					
NO.	DATE	REVISIONS	BY	APPR.	



MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT

PLAN AND PROFILE  
LINE "A", STA. 25+25 TO STA. 34+50

JOB NO. 20024530.1 DRAWN RWP/MOB SCALE: HORIZ.: 1"=50' VERT.: 1"=5'  
DATE 8/03 DESIGNED AJS/GLM APPROVED AJS



2801 S. PENNSYLVANIA STREET  
INDIANAPOLIS, INDIANA 46225 (317) 786-0461

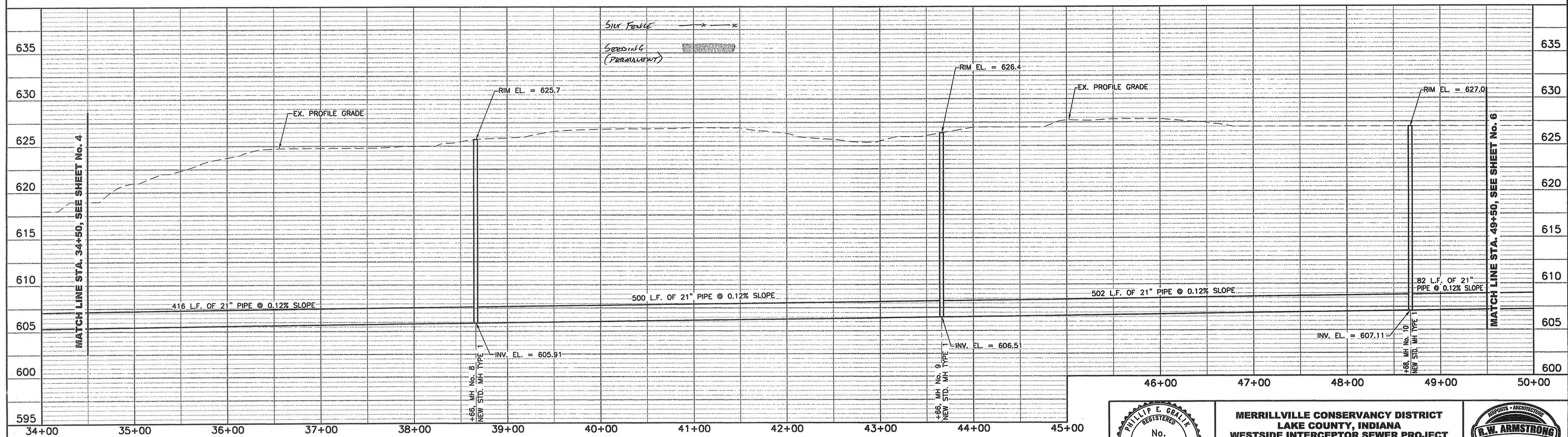
SHEET 4 OF 14



WOODED AREA EVERYWHERE

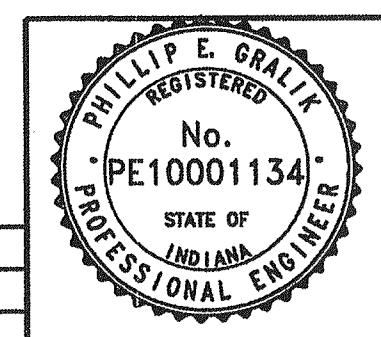


PLAN



PROFILE

NO.	DATE	REVISIONS	BY	APPR.
3				
2				
1				



**MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT**

**PLAN AND PROFILE  
LINE "A", STA. 34+50 TO STA. 49+50**

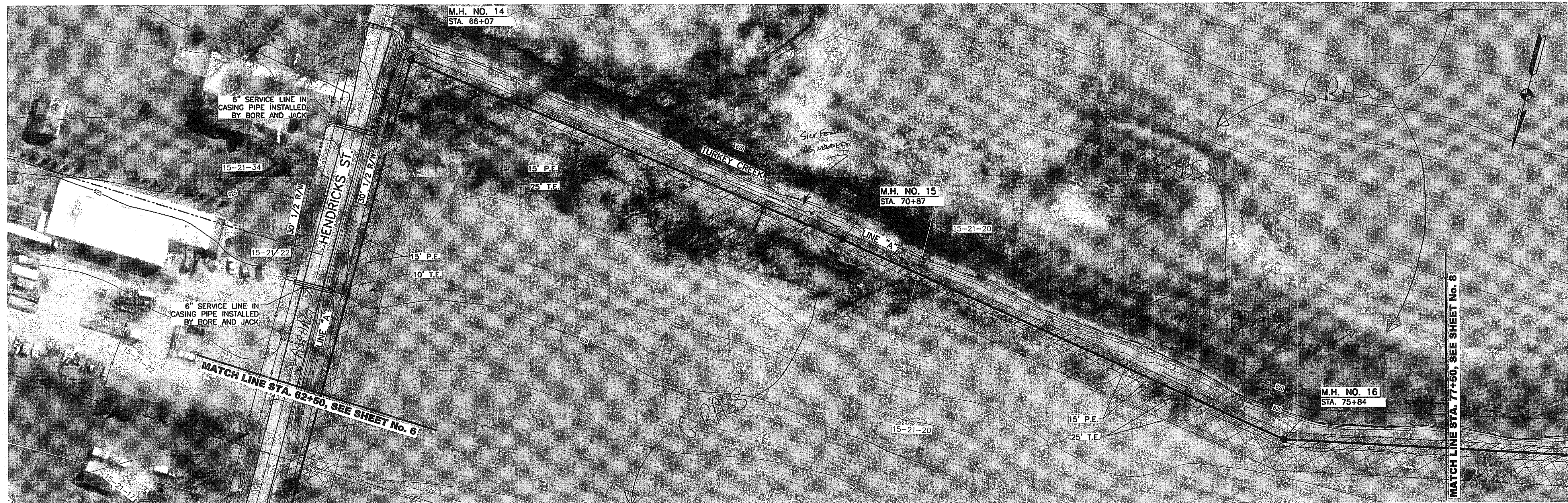
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DATE 8/03	DESIGNED AJS/GLM	APPROVED AJS



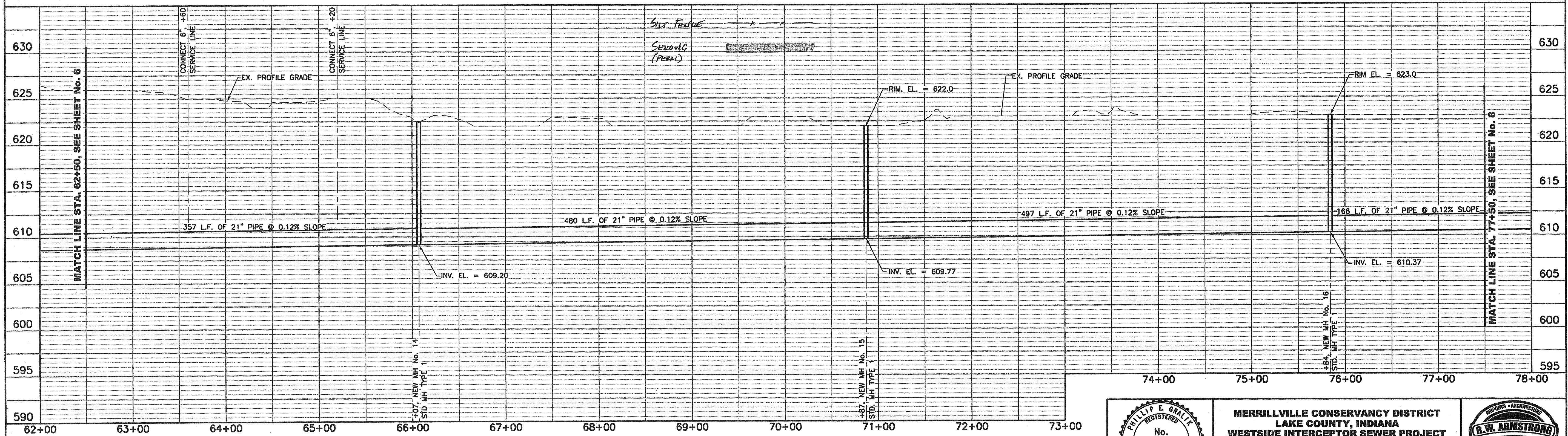






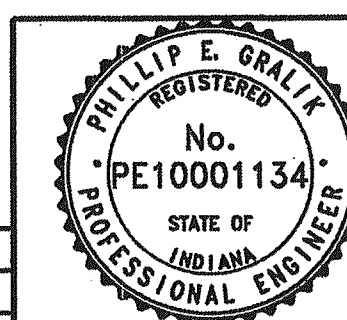


PLAN



PROFILE

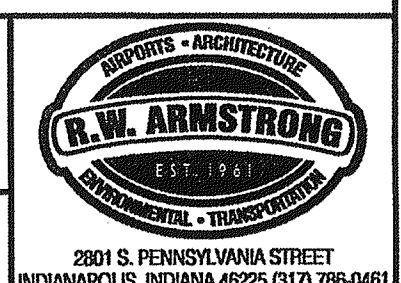
3					
2					
1					
NO.	DATE	REVISIONS	BY	APPR.	



MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT

PLAN AND PROFILE  
STA. 62+50 TO STA. 77+50

JOB NO. 20024530.1 DRAWN RWP/MOB SCALE: HORIZ: 1"=50' VERT: 1"=5'  
DATE 8/03 DESIGNED AJS/GLM APPROVED AJS



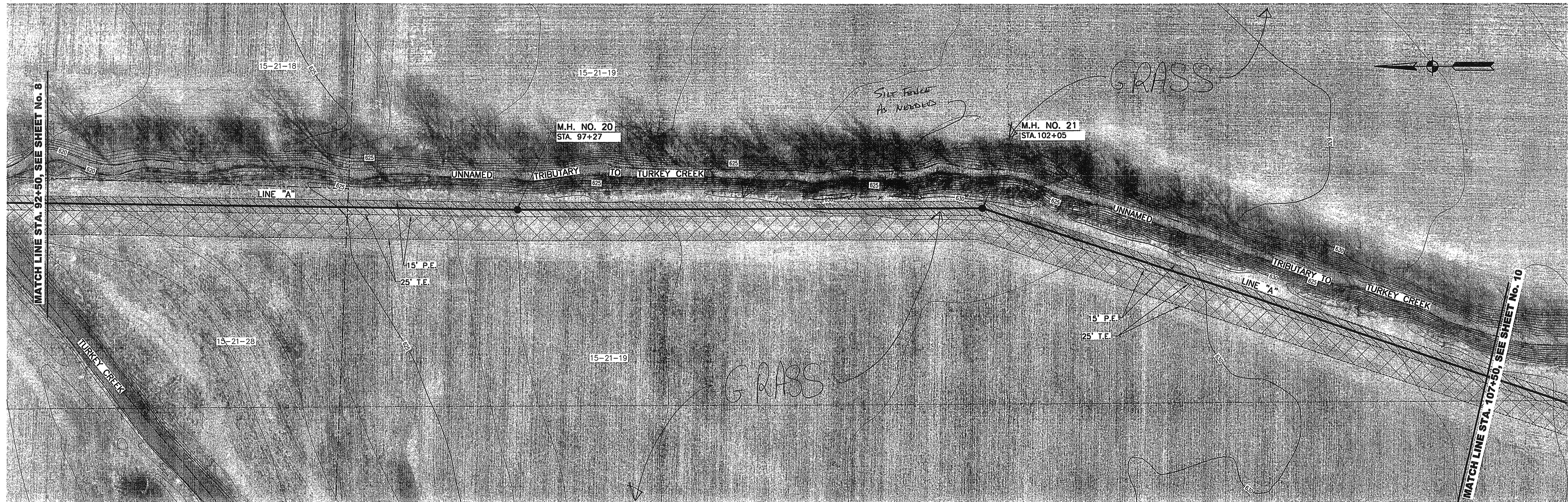
2801 S. PENNSYLVANIA STREET  
INDIANAPOLIS, INDIANA 46226 (317) 786-0461

SHEET 7 OF 14

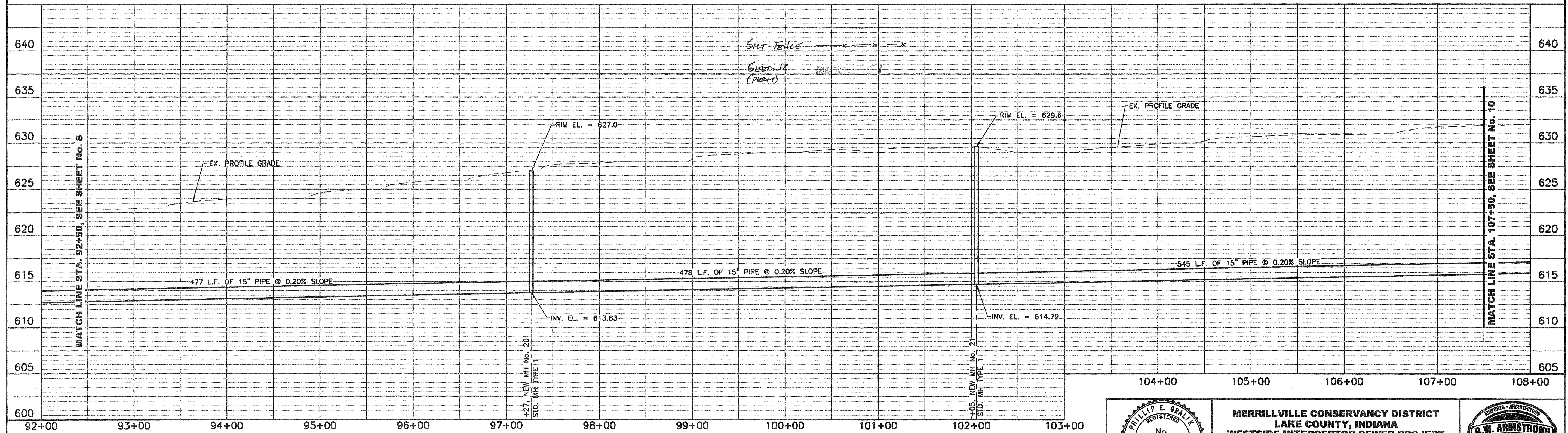






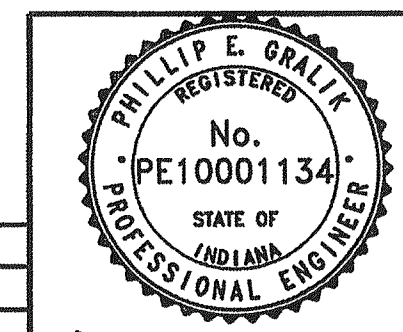


PLAN



PROFILE

3					
2					
1					
NO.	DATE	REVISIONS	BY	APPR.	



MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT

PLAN AND PROFILE  
STA. 92+50 TO STA. 107+50

JOB NO. 20024530.1	DRAWN RWP/MOB	SCALE: HORIZ. 1"=50'
DATE 8/03	DESIGNED AJS/GLM	VERT. 1"=5'
	APPROVED AJS	



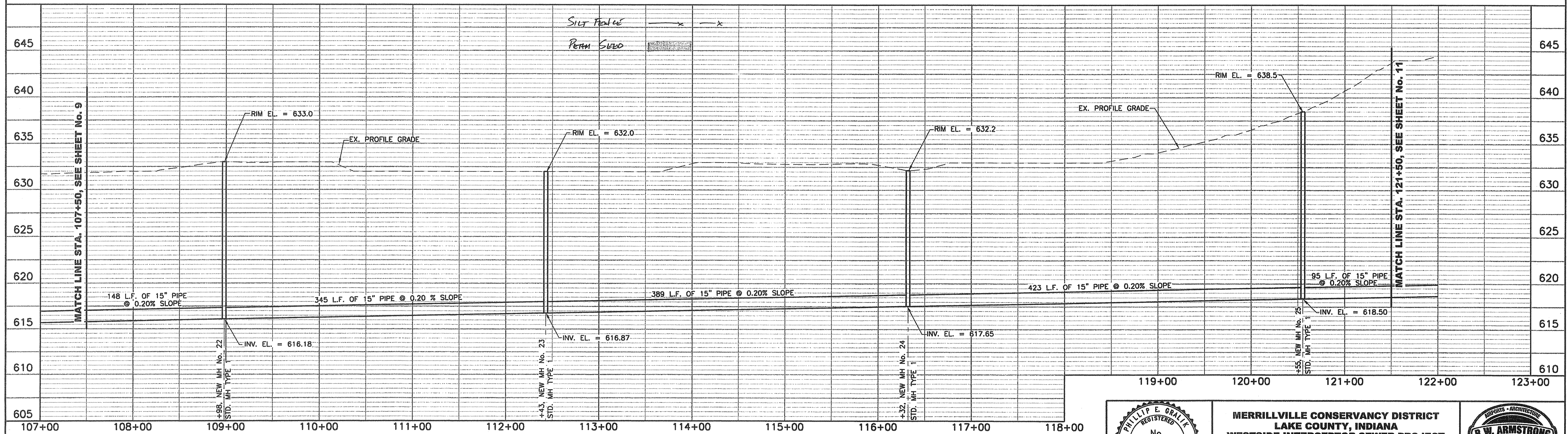
2801 S. PENNSYLVANIA STREET  
INDIANAPOLIS, INDIANA 46225 (317) 786-9461

SHEET 9 OF 14



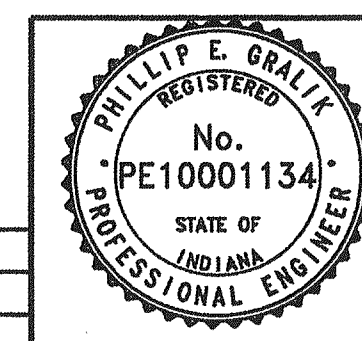


PLAN



PROFILE

NO.	DATE	REVISIONS	BY	APPR.
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MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT

PLAN AND PROFILE  
STA. 107+50 TO STA. 121+50

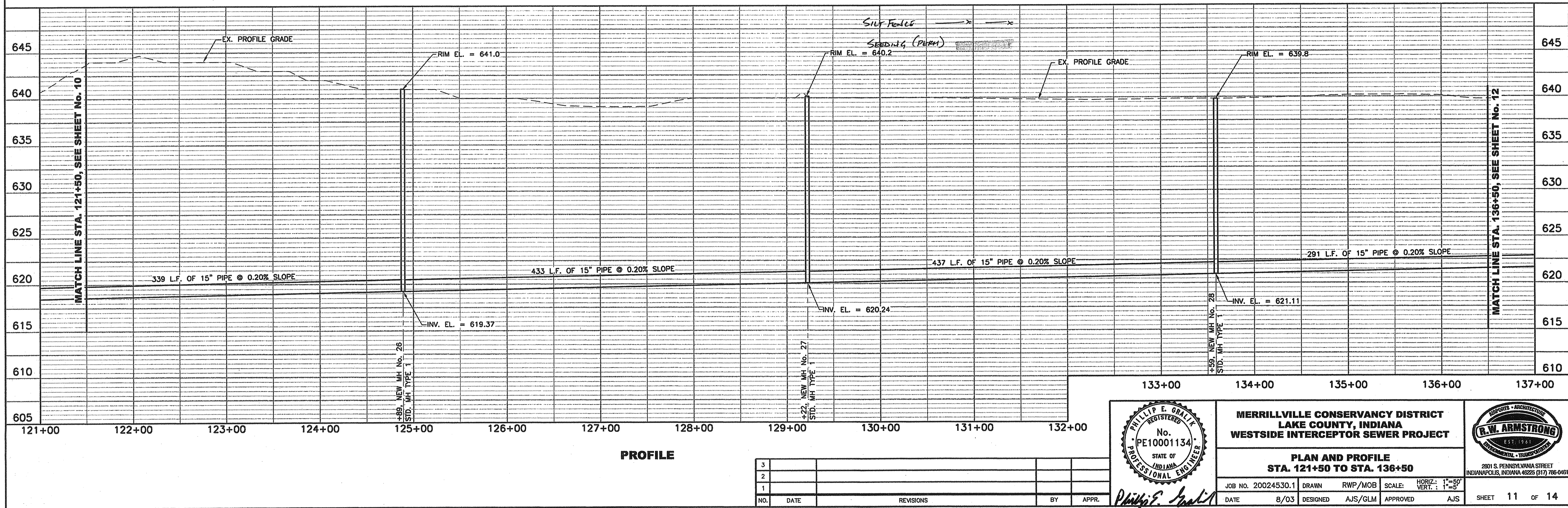
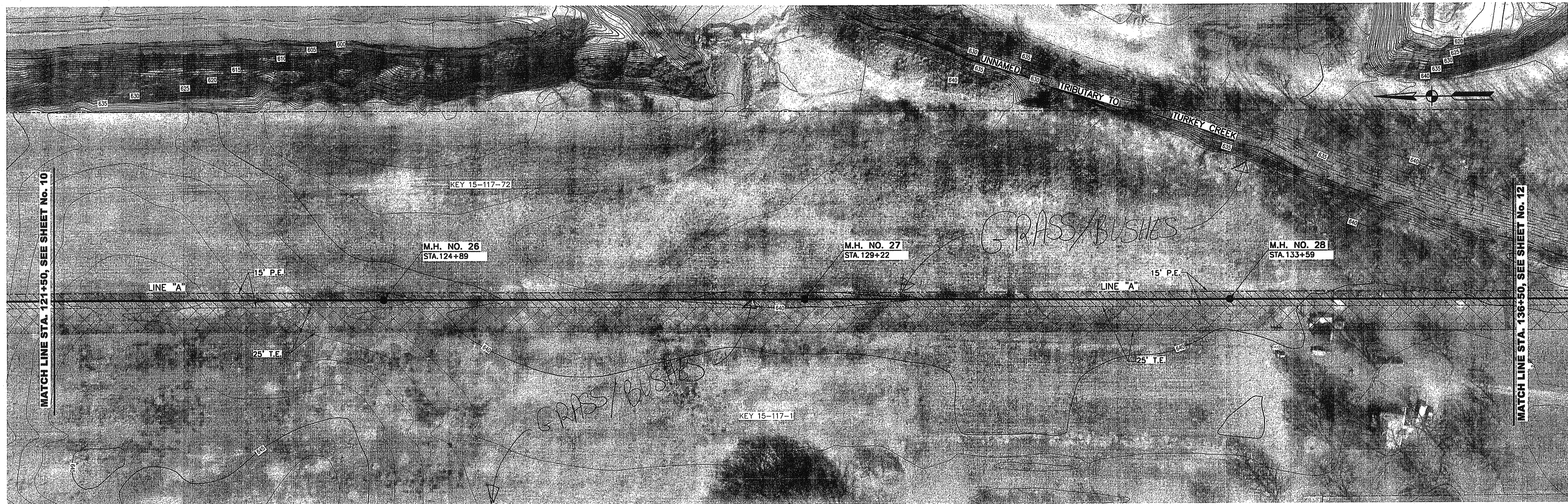
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DATE 8/03 DESIGNED AJS/GLM APPROVED AJS



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SHEET 10 OF 14





NO.	DATE	REVISIONS	BY	APPR.
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1				



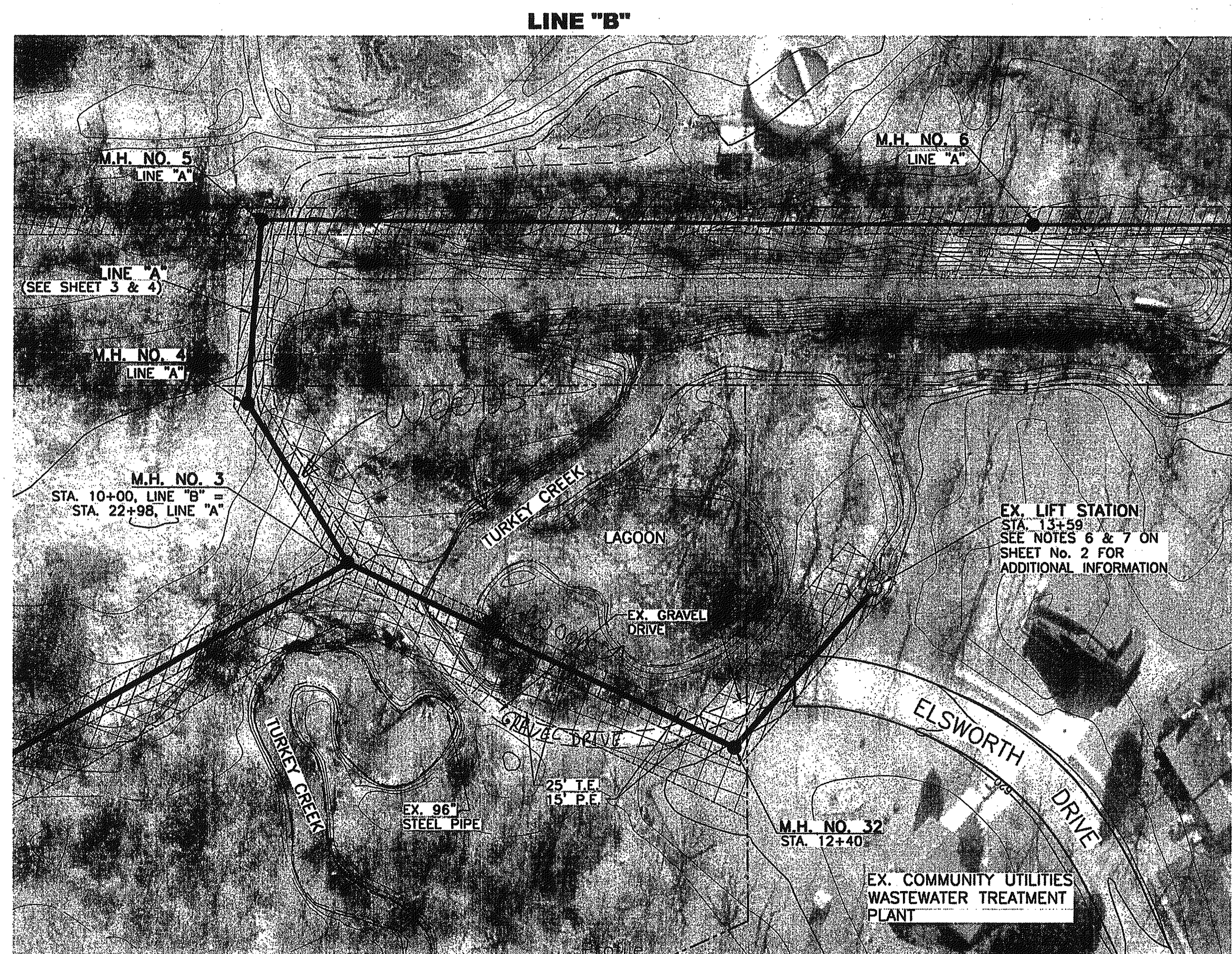
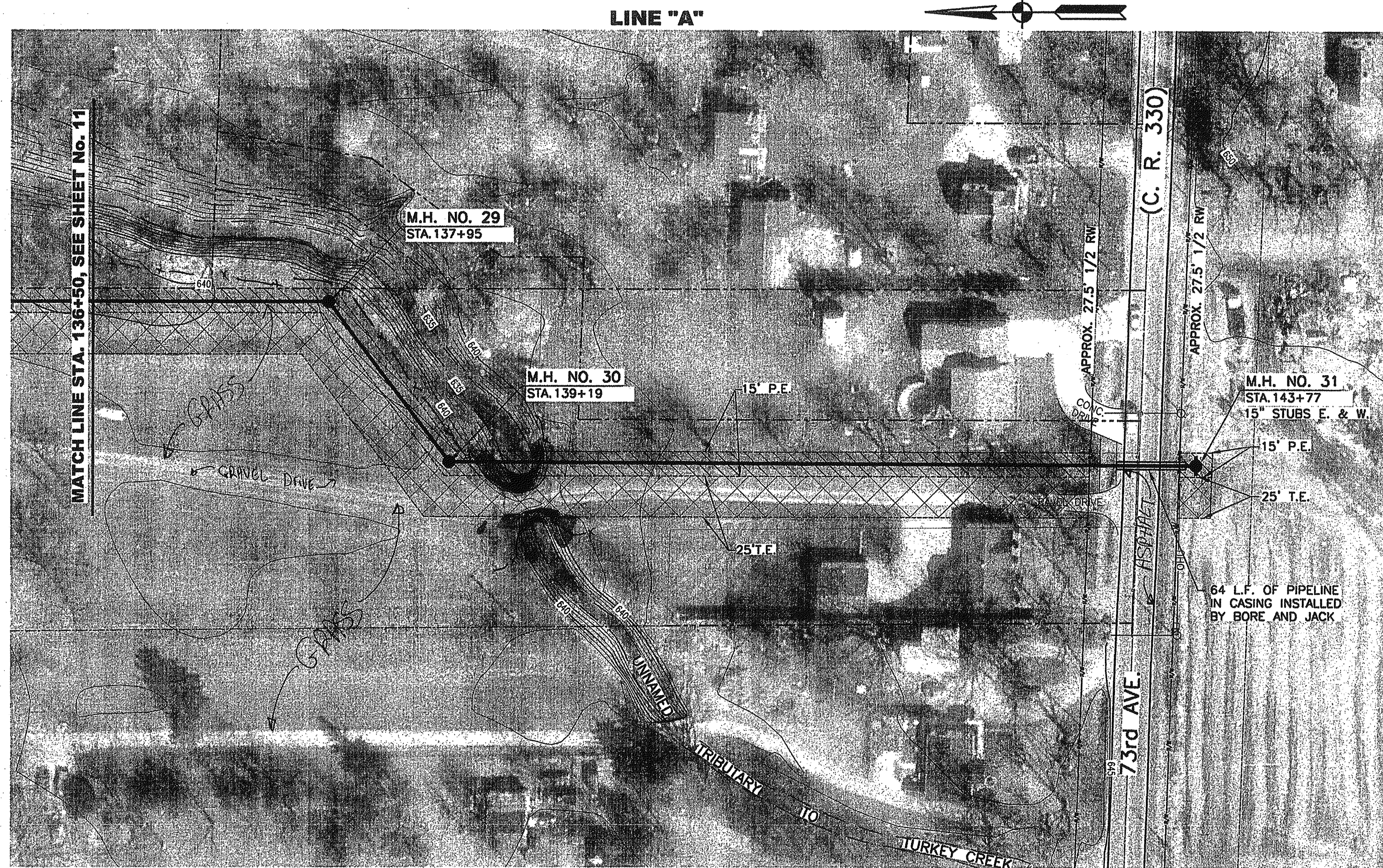
**MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT**

**PLAN AND PROFILE  
STA. 121+50 TO STA. 136+50**

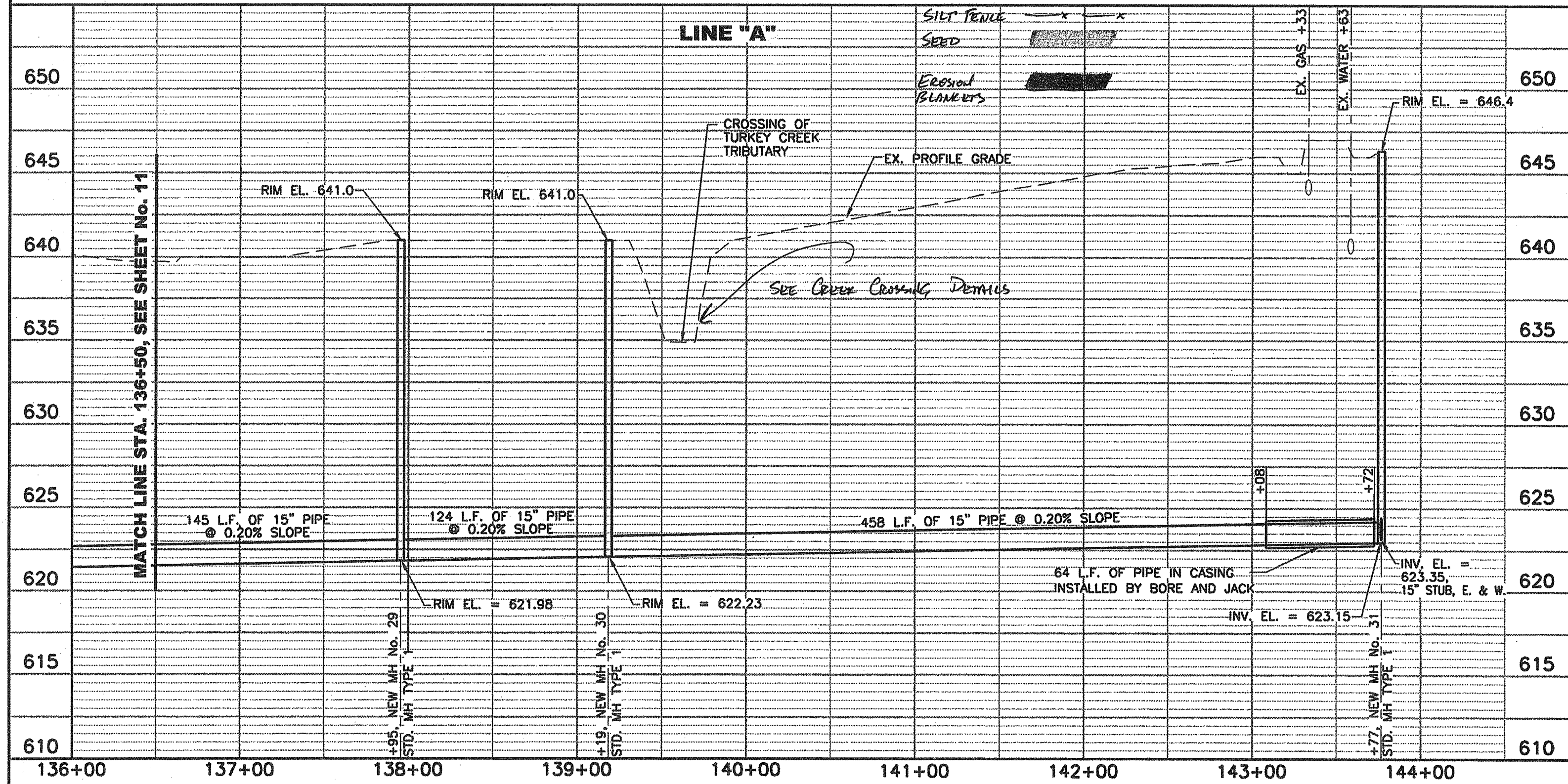
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DATE 8/03 DESIGNED AJS/GLM APPROVED AJS VERT.: 1"=5'



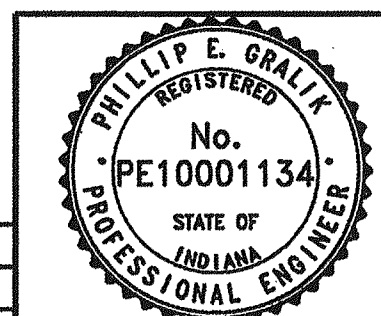
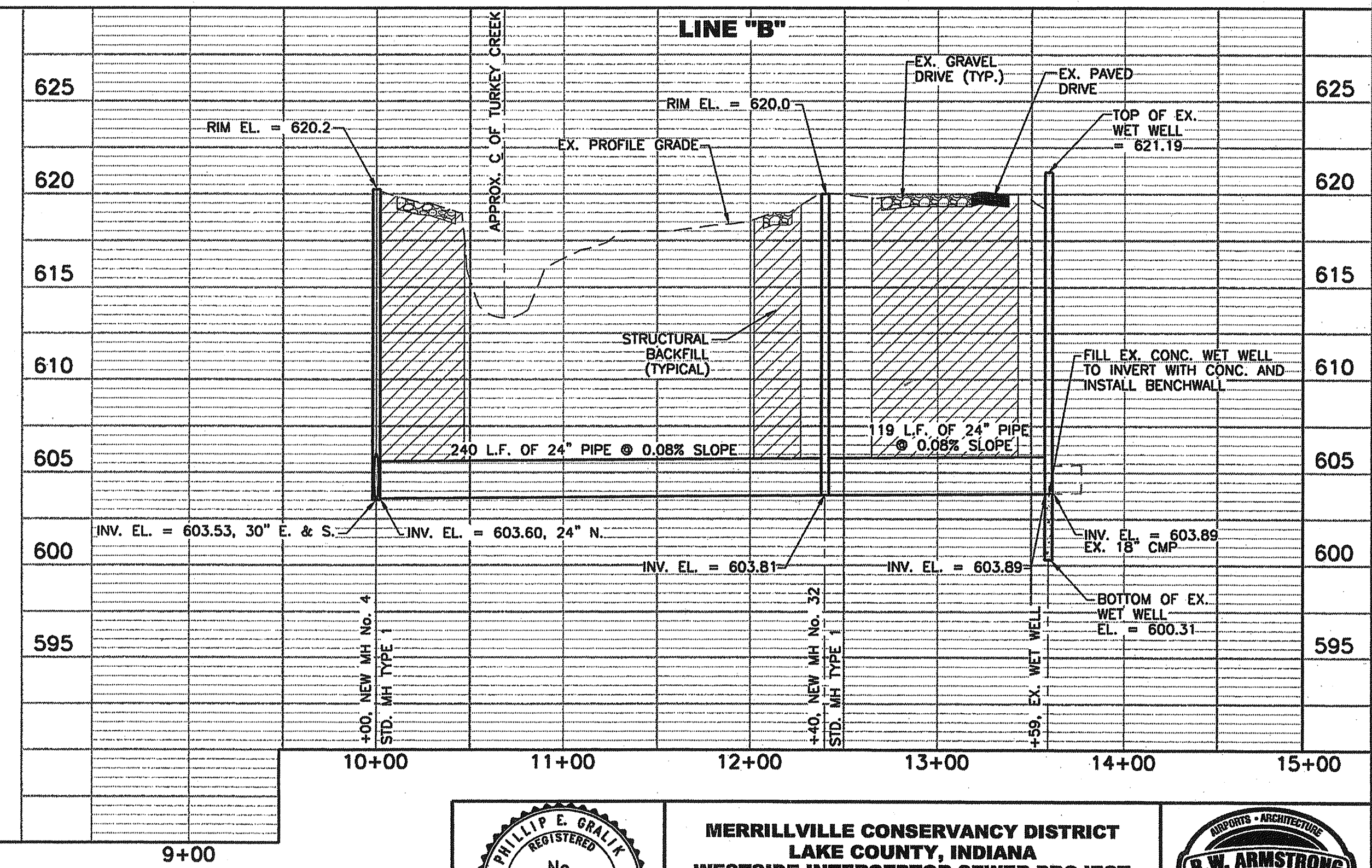




PLAN



PROFILE



MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT



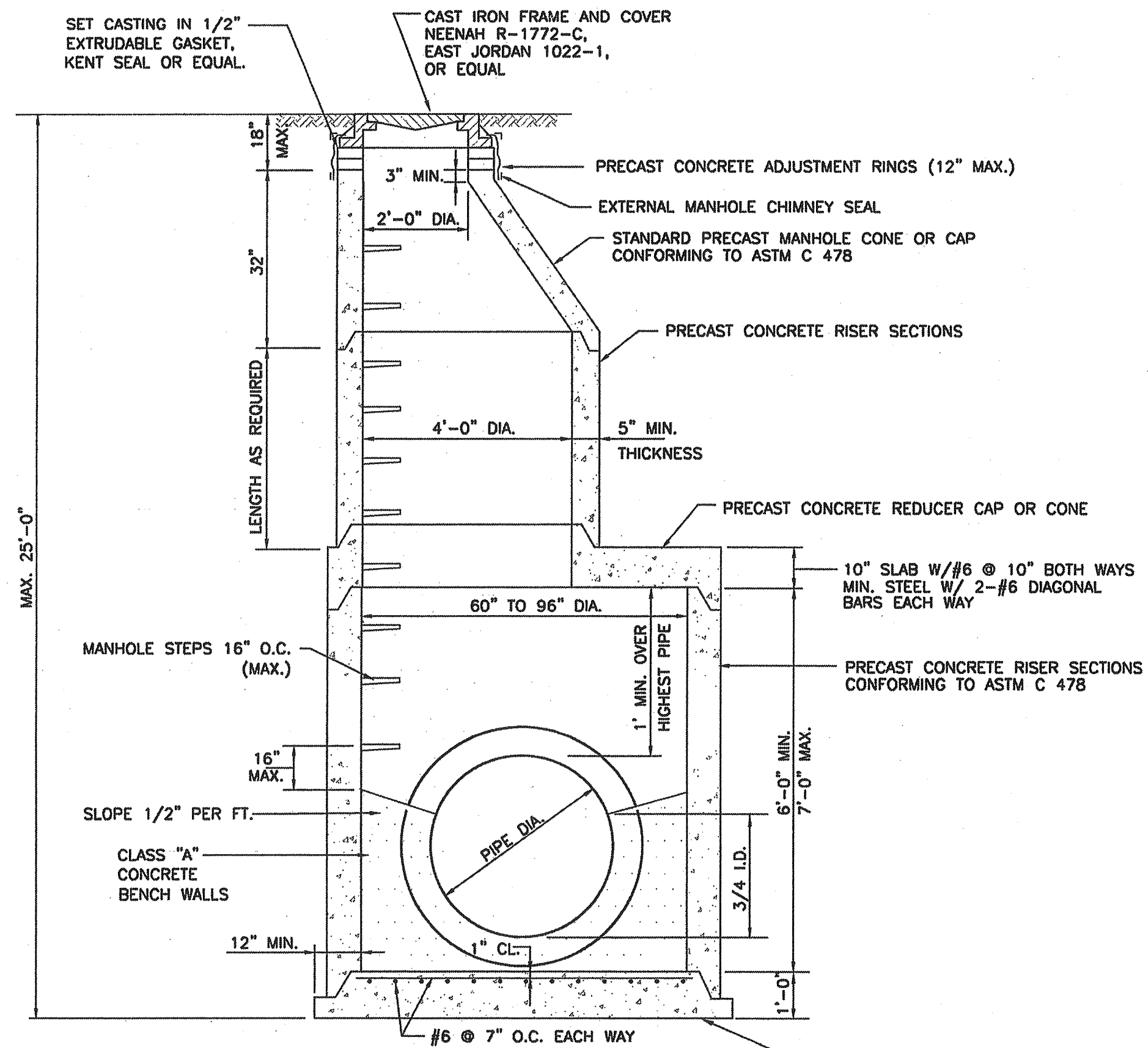
PLAN AND PROFILE  
LINE "A" - STA. 136+50 TO STA. 143+77 & LINE "B"

JOB NO. 20024530.1 DRAWN: RWP/MOB SCALE: HORIZ: 1"=50' VERT: 1"=5'  
DATE: 8/03 DESIGNED: AJS/GLM APPROVED: AJS

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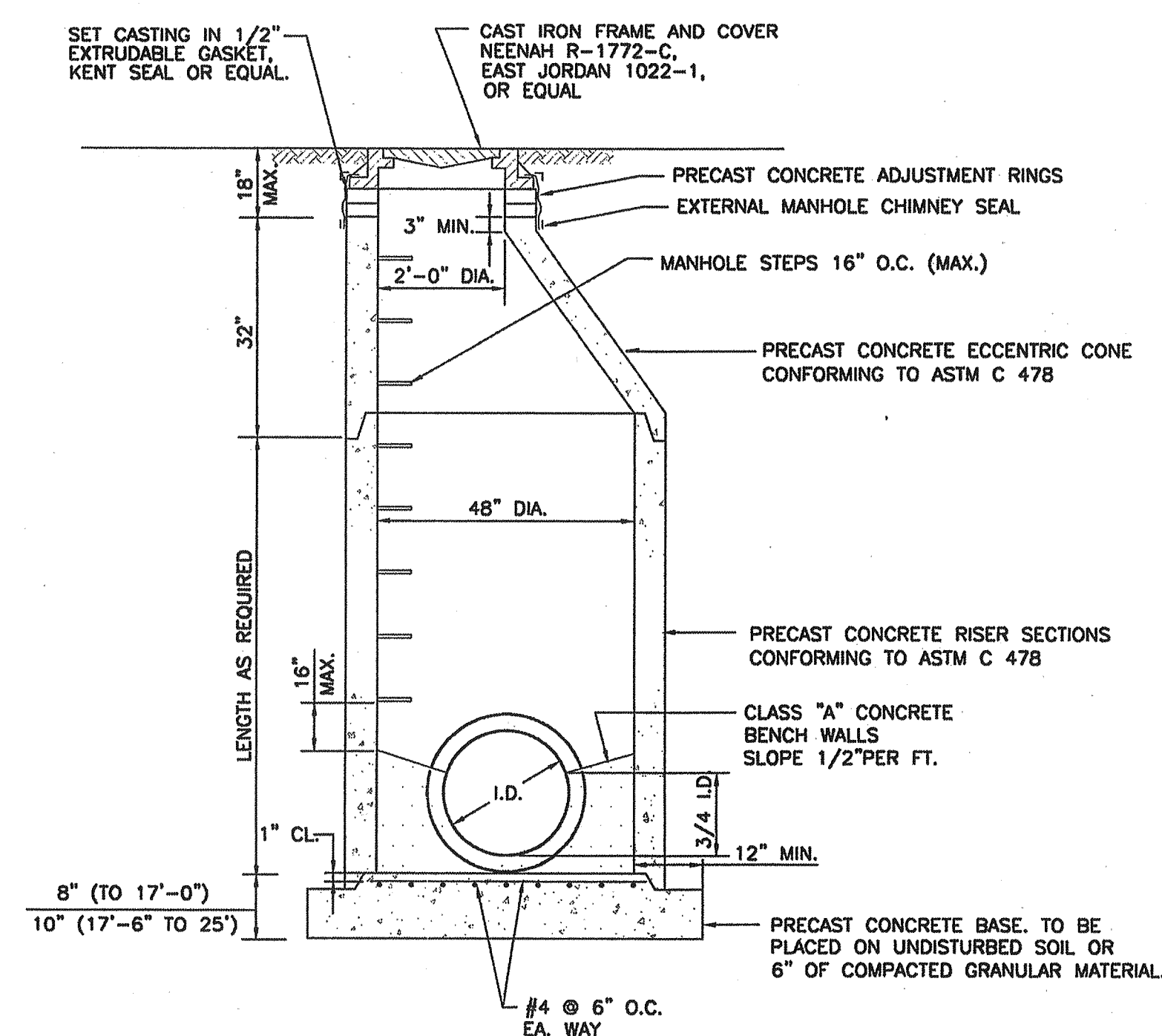
SHEET 12 OF 14





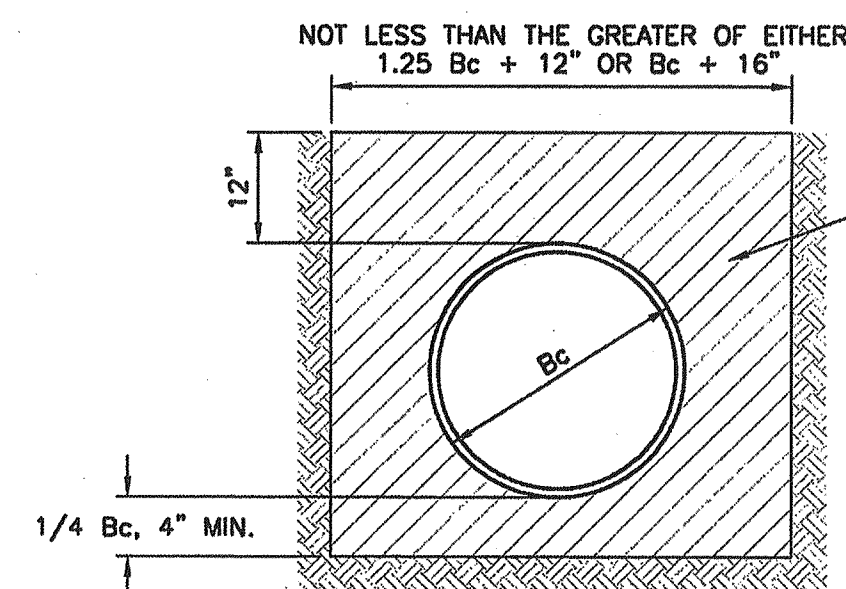
**STANDARD PRECAST CONCRETE MANHOLE - TYPE 2**  
SEE TABLE FOR MAXIMUM INSIDE DIAMETER FOR CONNECTING SEWER  
NOT TO SCALE

INSIDE MANHOLE DIAMETER, INCHES	SEWER PIPE DEFLECTION THROUGH MANHOLE, DEGREES	TYPE OF SEWER PIPE	
		PVC	D/F FIBER
60"	0° - 45°	36"	36"
60"	46° - 90°	27"	24"
72"	0° - 45°	42"	42"
72"	46° - 90°	30"	30"
84"	0° - 45°	48"	48"
84"	46° - 90°	36"	36"
96"	0° - 45°	42"	42"
96"	46° - 90°	48"	48"



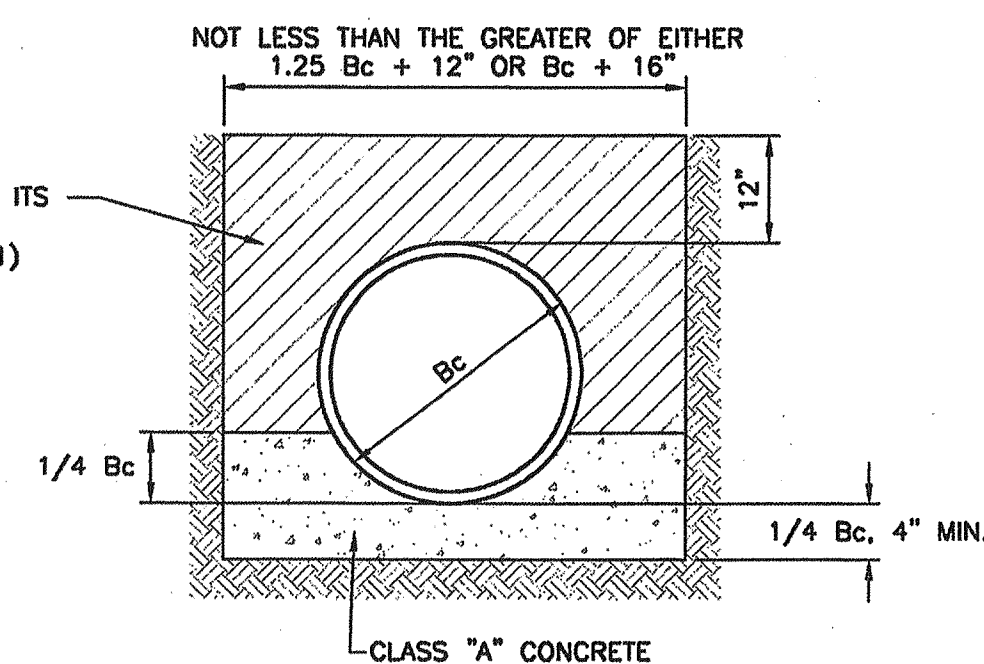
**STANDARD PRECAST CONCRETE MANHOLE - TYPE 1**  
SEE TABLE FOR MAXIMUM INSIDE DIAMETER FOR CONNECTING SEWER  
NOT TO SCALE

INSIDE MANHOLE DIAMETER, INCHES	SEWER PIPE DEFLECTION THROUGH MANHOLE, DEGREES	TYPE OF SEWER PIPE	
		PVC	D/F FIBER
48"	0° - 45°	27"	24"
48"	46° - 90°	21"	20"



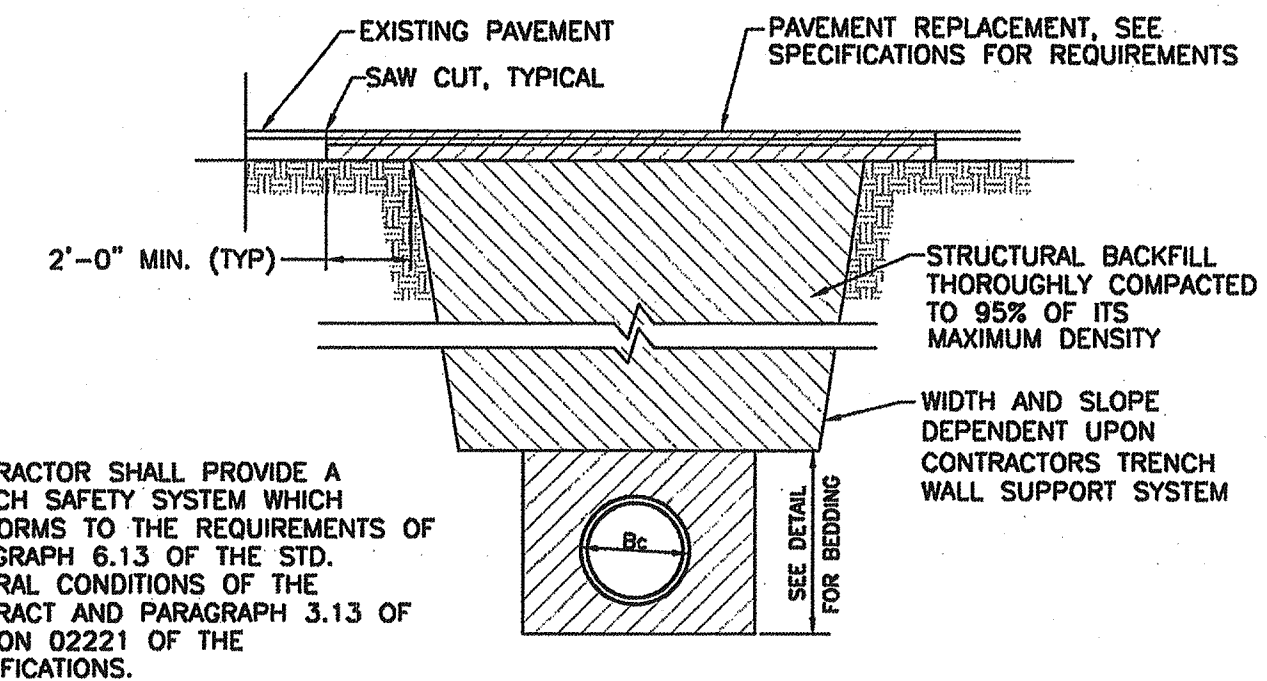
NOTE: IN ROCK TRENCH, EXCAVATE AT LEAST 6" BELOW THE BELL OF THE PIPE.

**STANDARD BEDDING**



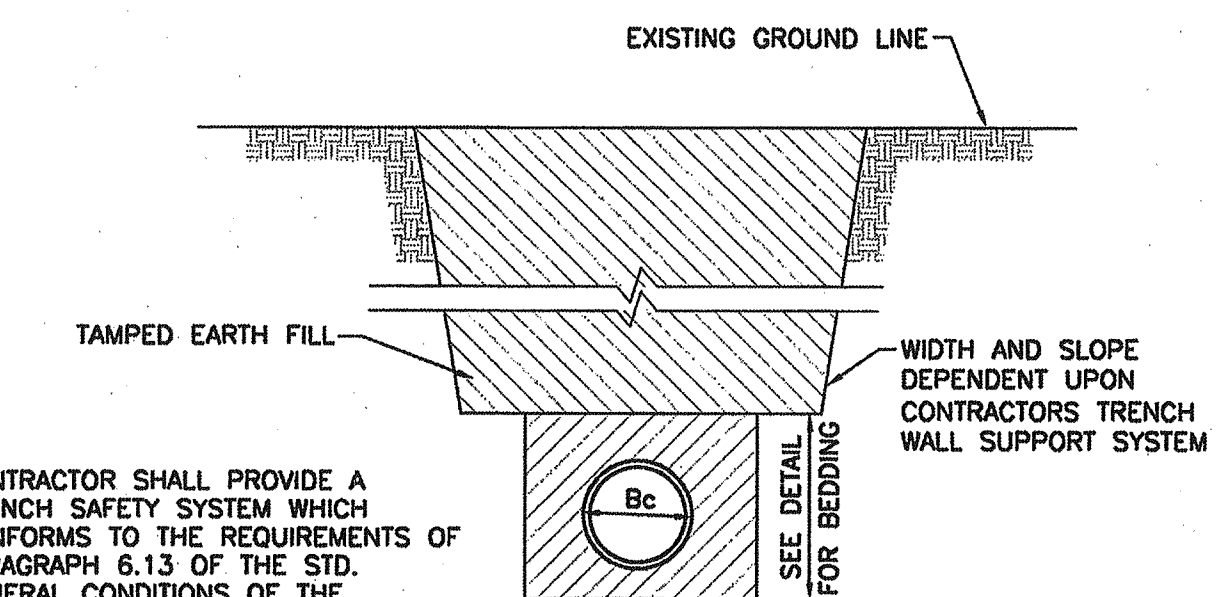
**CONCRETE CRADLE**

**BEDDING DETAILS**

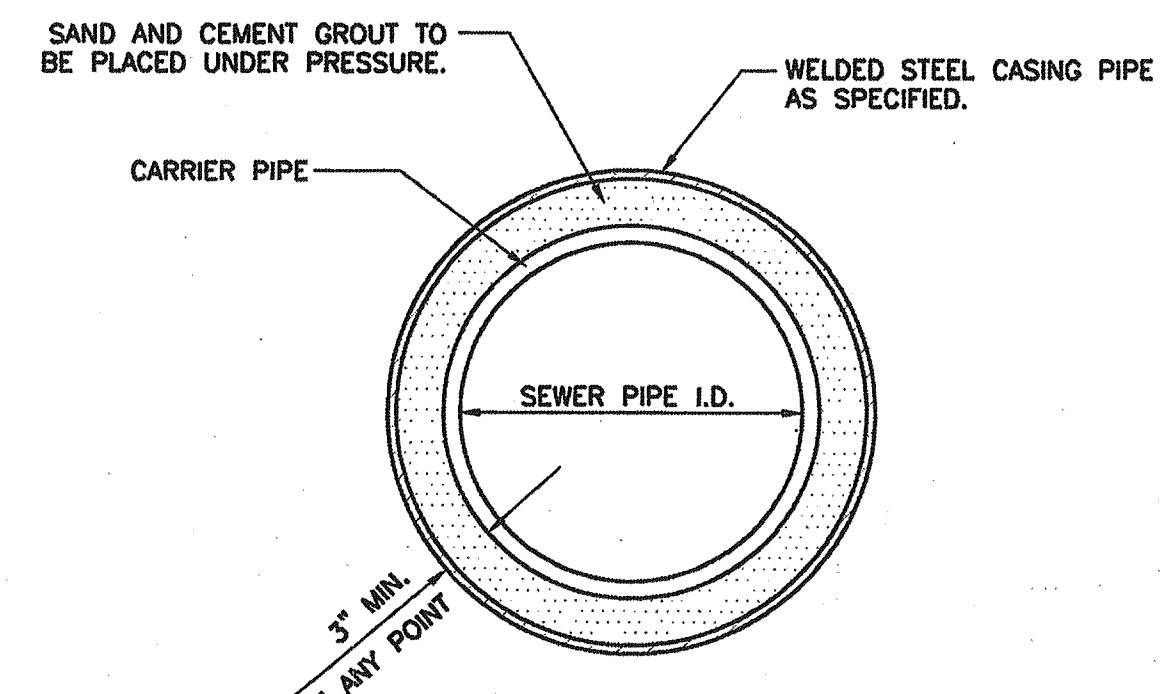


**TRENCH AND BACKFILL UNDER PAVEMENT DETAIL AND OTHER LOCATIONS\***

\* AS SPECIFIED IN SECTION 02221 OF THE SPECIFICATIONS OR WHERE STRUCTURAL BACKFILL IS REQUIRED BY PLANS.

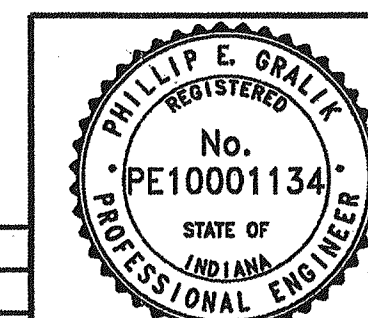


**TRENCH AND BACKFILL DETAIL**



**CROSS-SECTION PIPE IN CASING**

NO.	DATE	REVISIONS	BY	APPR.
3				
2				
1				



**MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT**

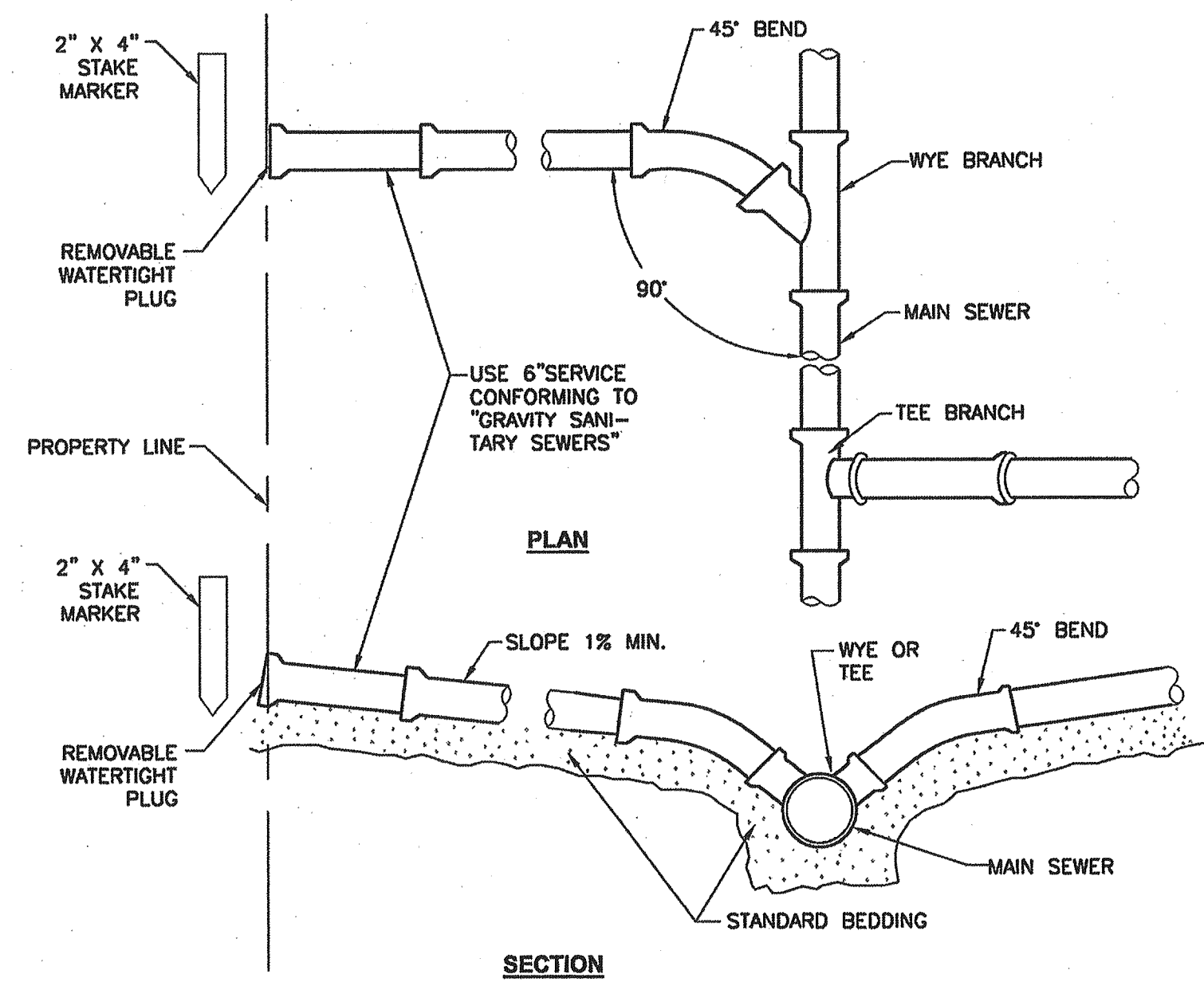
**MISCELLANEOUS DETAILS**

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DATE 8/03	DESIGNED AJS/GLM	APPROVED AJS

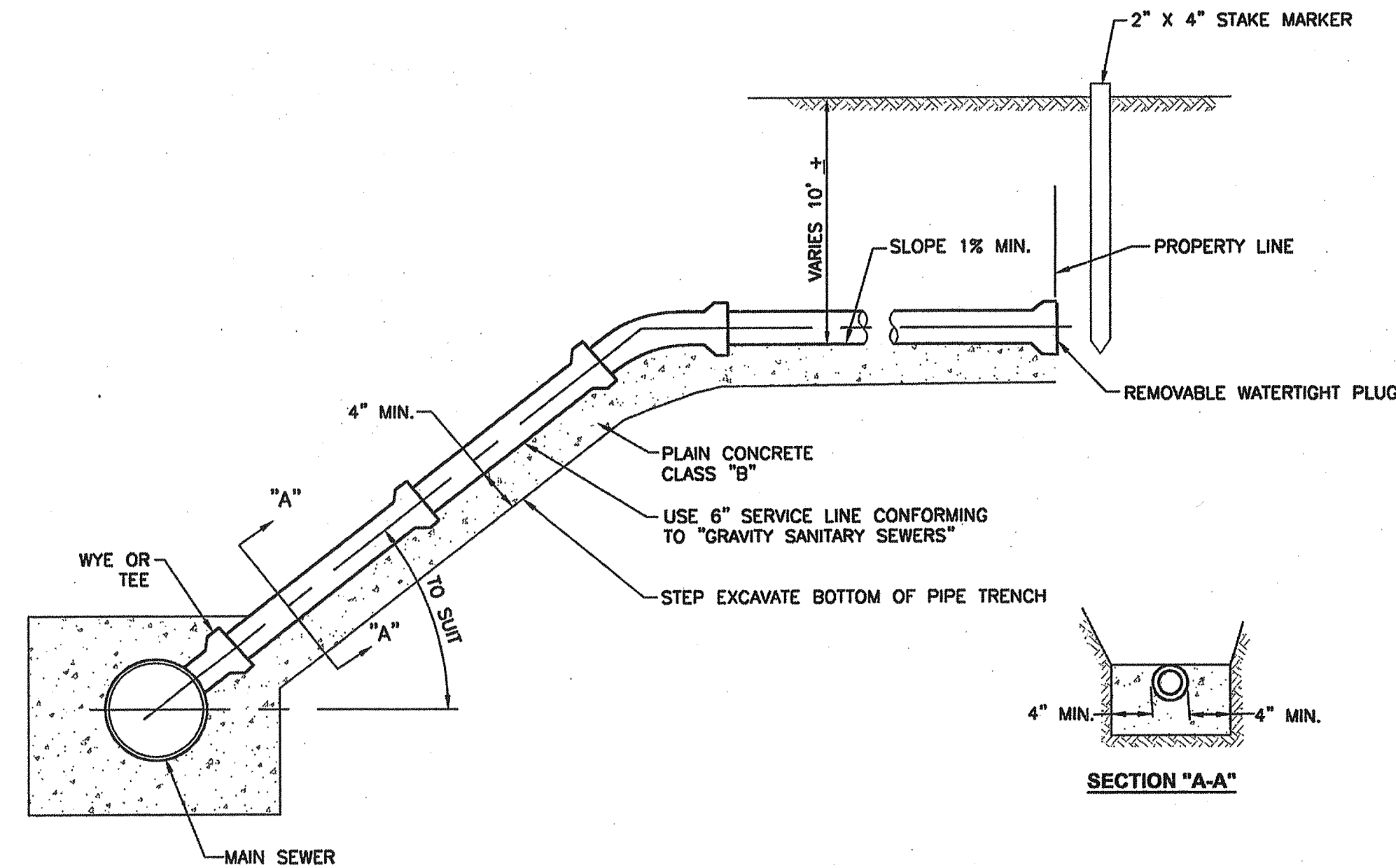


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INDIANAPOLIS, INDIANA 46225 (317) 766-0461

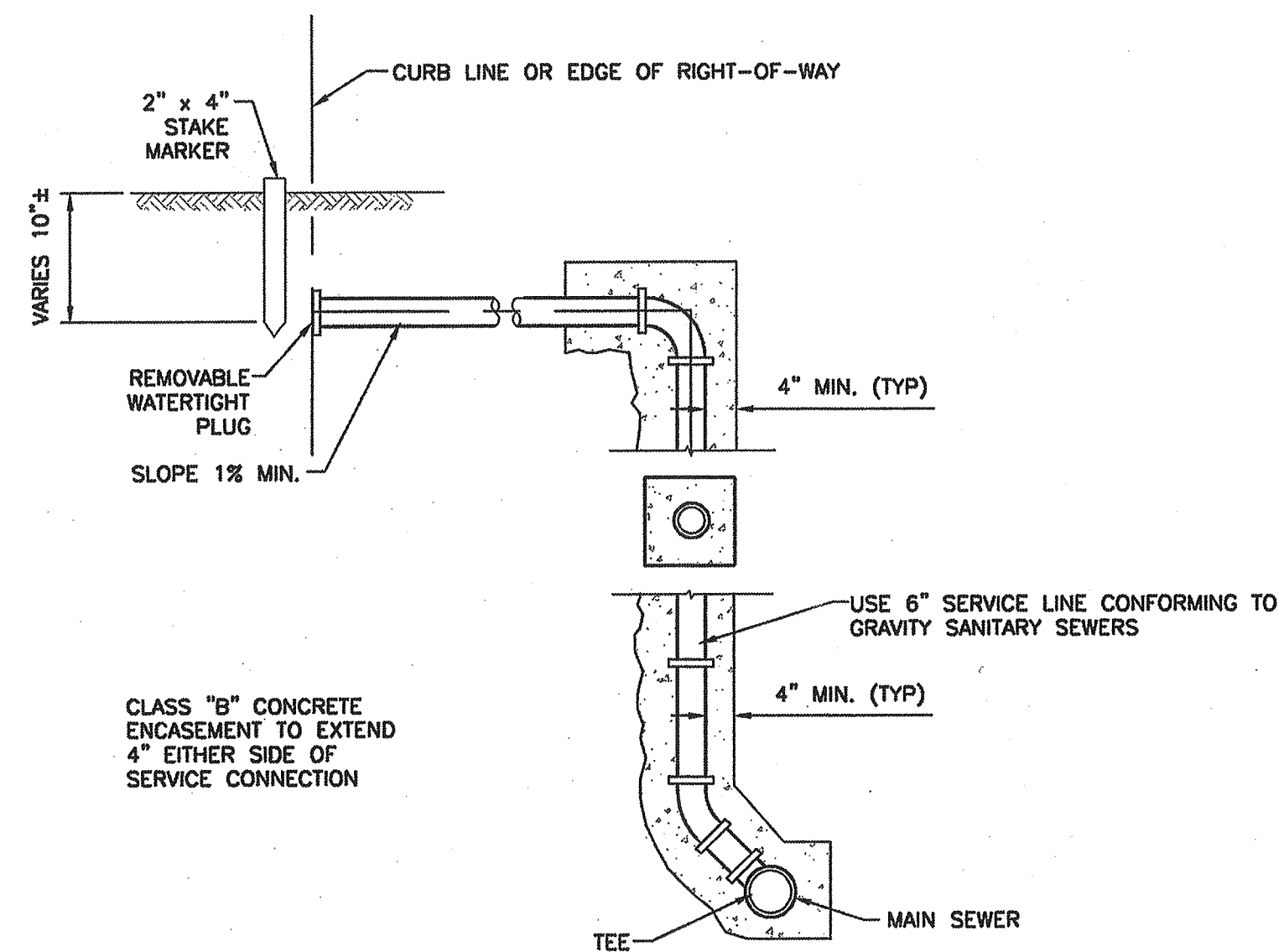




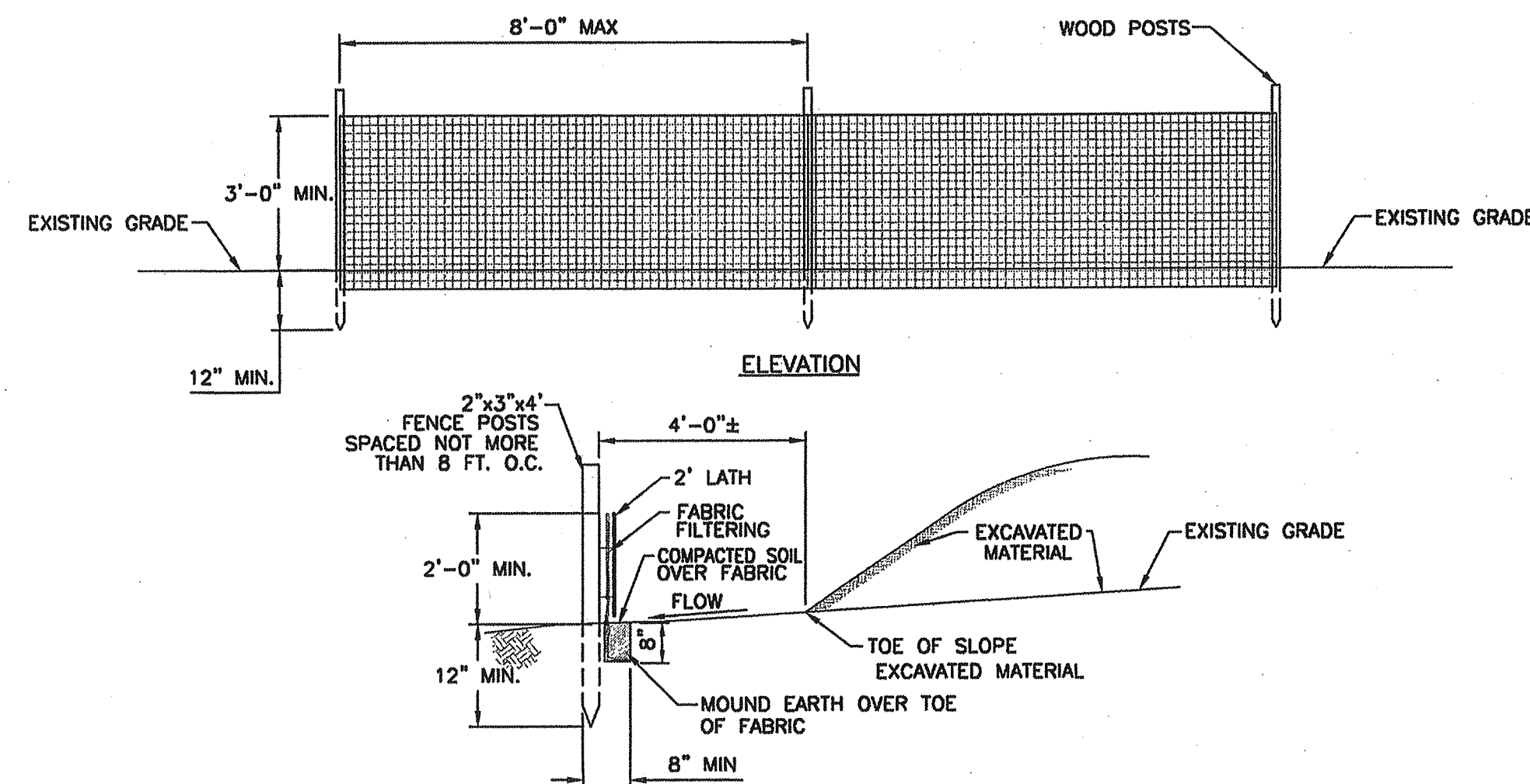
**SERVICE CONNECTION - SHALLOW**



**SERVICE CONNECTION - DEEP**

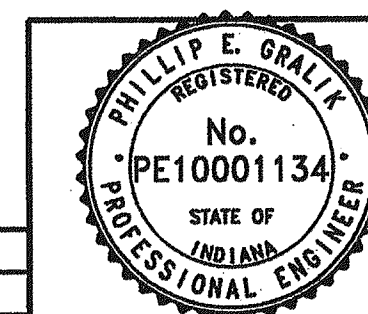


**SERVICE CONNECTION - DEEP  
(LIMITED R/W ONLY)**



**UNSUPPORTED SILT FENCE TYPE A  
NOT TO SCALE**

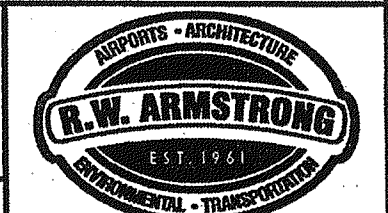
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NO.	DATE	REVISIONS	BY	APPR.	



**MERRILLVILLE CONSERVANCY DISTRICT  
LAKE COUNTY, INDIANA  
WESTSIDE INTERCEPTOR SEWER PROJECT**

**MISCELLANEOUS DETAILS**

JOB NO. 20024530.1	DRAWN	SCALE:	AS NOTED
DATE 8/03	DESIGNED	AJS/GLM	APPROVED



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**APPENDIX M**

**MODELING ANALYSIS**





## Memorandum

*To: James M. Czarnik, Senior Engineer  
Robinson Engineering*

*From: Mandeera Wagle, Project Manager  
Matt Rembold, Project Engineer  
CDM*

*Date: November 15, 2010*

*Subject: Taft Street Lift Station Analysis*

The Merrillville Conservancy District (MCD) owns and operates the Taft Street Lift Station, which is located near the intersection of 63<sup>rd</sup> Avenue and State Road 55, in Merrillville, Indiana. The lift station consists of a wet well, four pumps, and two forcemains that operate under pumped conditions only (i.e., the forcemains do not flow by gravity in case the pumps are off). Based on information provided by Robinson Engineering, the lift station currently pumps approximately 3.2 MGD average daily flow and 10 MGD peak flow, although flows through the lift station have been known to reach 12 MGD. MCD has plans to expand the station to a capacity of 25 MGD in the next few months.

Over the weekend of October 15<sup>th</sup> through the 17<sup>th</sup>, 2010, it is believed that a compressor in the lift station failed, preventing the pumps from operating and causing the water level in the station to rise significantly. High water marks were found within the wet well at approximately 615 feet, 25 feet above the elevation of the bottom slab of the lift station. Subsequent to the lift station failure, a fish kill in nearby Turkey Creek occurred.

Robinson Engineering engaged CDM to perform a static analysis of the Taft Street Lift Station to estimate the degree and extent of sewer system surcharging under various flow scenarios, including a pump station failure condition. This memorandum summarizes the methodology and results of the analysis, and contains the following sections:

- Model Development
- Simulated Scenarios
- Results and Conclusions



## Section 1: Model Development

The analysis of the Taft Street Lift Station was based on a computer model of the sewer system immediately upstream of the lift station. Based on the objective of this analysis, the model extents were limited to the area containing manholes with rim elevations just at the 615 foot high water elevation. Figure 1 presents the study area and model extents. The study area is bounded by 59<sup>th</sup> Place to the north, Cleveland Circle to the east, and the future Prairie Creek development to the south and west. Based on information provided by Robinson Engineering, the sewers within the study area range from 8 inches to 48 inches in diameter, with the main Taft Street interceptor being 48 inches in diameter. On average, the system is approximately 12 feet below grade.

The model was developed based on AutoCAD drawings (XR-Sewers.dwg and Master.dwg) provided by Robinson Engineering. Spatial referencing and pipe and manhole attribute information was converted from these AutoCAD drawings first to ArcGIS and then imported to a SWMM-based modeling software package. The following attribute information was required to develop the model:

- Manholes: x-coordinate, y-coordinate, rim elevation, invert elevation
- Pipes: upstream and downstream invert elevation, diameter, pipe length

All attribute information was populated using the AutoCAD maps provided. It was assumed that the coordinate system of the AutoCAD drawings is North American Datum (NAD) 1983 State Plane Indiana West, in units of feet. Missing rim elevations were estimated using a publically available 10-meter digital elevation model (DEM), and missing invert elevations were estimated based on slope. If a slope could not be determined based on information provided on the source AutoCAD drawings then minimum slopes as provided by the Indiana Administrative Code, Title 327 Water Pollution Control Board 327 IAC 3-6-12 were used. All pipes were assumed to be circular in shape with a Manning's n of 0.013.

Because the analysis focused on a pump station failure condition, and because the forcemains from the lift station cannot flow under gravity conditions, the downstream boundary of the model was the Taft Street Lift Station wet well. The wet well was modeled as a storage node, based on the existing footprint (as shown on the provided AutoCAD drawings) and inverts (as shown on the Taft Street Lift Station Expansion to 25 MGD D-5 Section A drawing) with an outlet that prevented any flow from leaving the lift station.

It should be noted that for the purposes of this analysis, only the hydraulic network of the sewer system was built in to the model. Hydrology (i.e., sewersheds and associated



attributes,) and any impacts from inflow and infiltration was not required for the analysis and therefore was not included in the model.

## **Simulated Scenarios**

The model was used to evaluate three flow scenarios. The main scenario of interest was the flow condition that was occurring in the system during the weekend of the reported lift station failure. During this weekend, there was no rainfall, and the average daily dry weather flow of 3.2 MGD was assumed to have occurred through the system. Two additional flow scenarios, with 10 MGD peak flow and 12 MGD extreme flows, were also evaluated.

Flows were input as a direct, constant inflow at the wet well of the Taft Street Lift Station. System flows were then simulated over the October 14 through 20, 2010 period under static conditions (i.e., with no variability to the flow input). The model was used to analyze peak water level at the wet well, peak hydraulic grade line through the system, and any flooded manholes.

## **Results and Conclusions**

For all three simulated scenarios (3.2 MGD, 10 MGD, and 12 MGD flows), the model predicted only three manholes to flood. The three manholes are located in the vicinity of where the 36 inch diameter east-west aligned sewer just south of Turkey Creek enters the main north-south interceptor at Taft Street (see Figure 1). The peak level in the wet well was predicted to be approximately 617 feet for all three modeled scenarios. These results are in agreement with field investigations reported by Robinson Engineering, which indicated that the water level in the wet well reached approximately 615 feet, and flooding occurred at the same three manholes.

For all three simulated scenarios, the entire modeled system exhibited surcharged conditions to varying degrees (i.e., the peak hydraulic grade line was higher than the crown of the pipe). Figure 1 indicates portions of the system where the freeboard between the ground surface and the peak hydraulic grade line in the pipes is at least 8 feet. Figures 3 through 30 present profiles of various sections of the sewer system indicating the peak hydraulic grade line through that segment during the 3.2 MGD constant inflow / lift station failure scenario.

The results of this analysis indicate that for a lift station failure condition, it is likely that during average dry weather system flow conditions of 3.2 MGD the sewer system upstream of the Taft Street Lift Station will surcharge to varying degrees and manholes in the vicinity of Turkey Creek will flood to the ground surface.





# Taft Street Lift Station Analysis



Figure 1: Extent of Surcharging for 3.2 MGD  
Constant Flow / Pump Failure Scenario

N

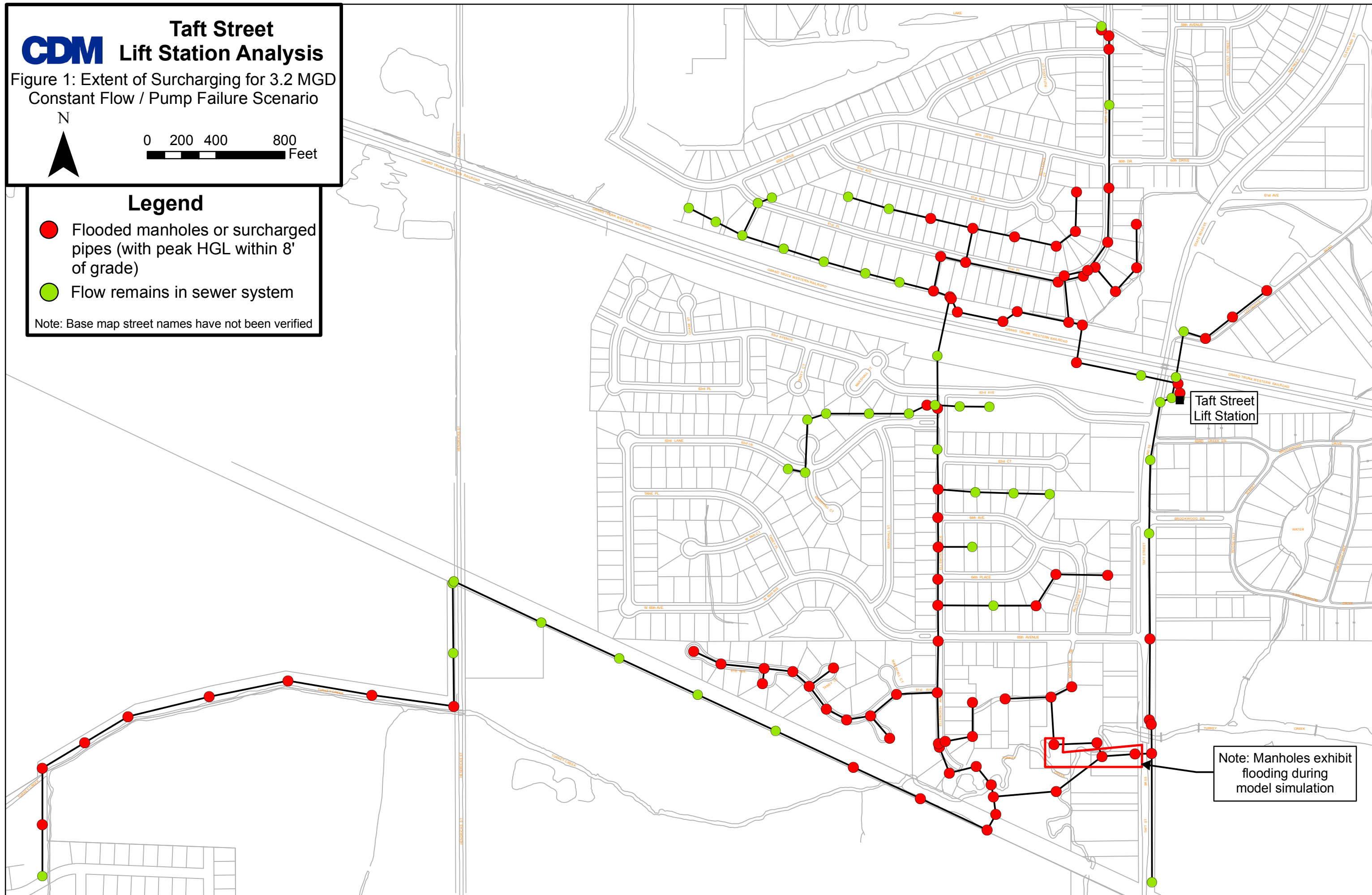


0 200 400 800  
Feet

## Legend

-  Flooded manholes or surcharged pipes (with peak HGL within 8' of grade)
-  Flow remains in sewer system

Note: Base map street names have not been verified



Taft Street  
Lift Station

Note: Manholes exhibit  
flooding during  
model simulation




### Figure 2: Profile Index

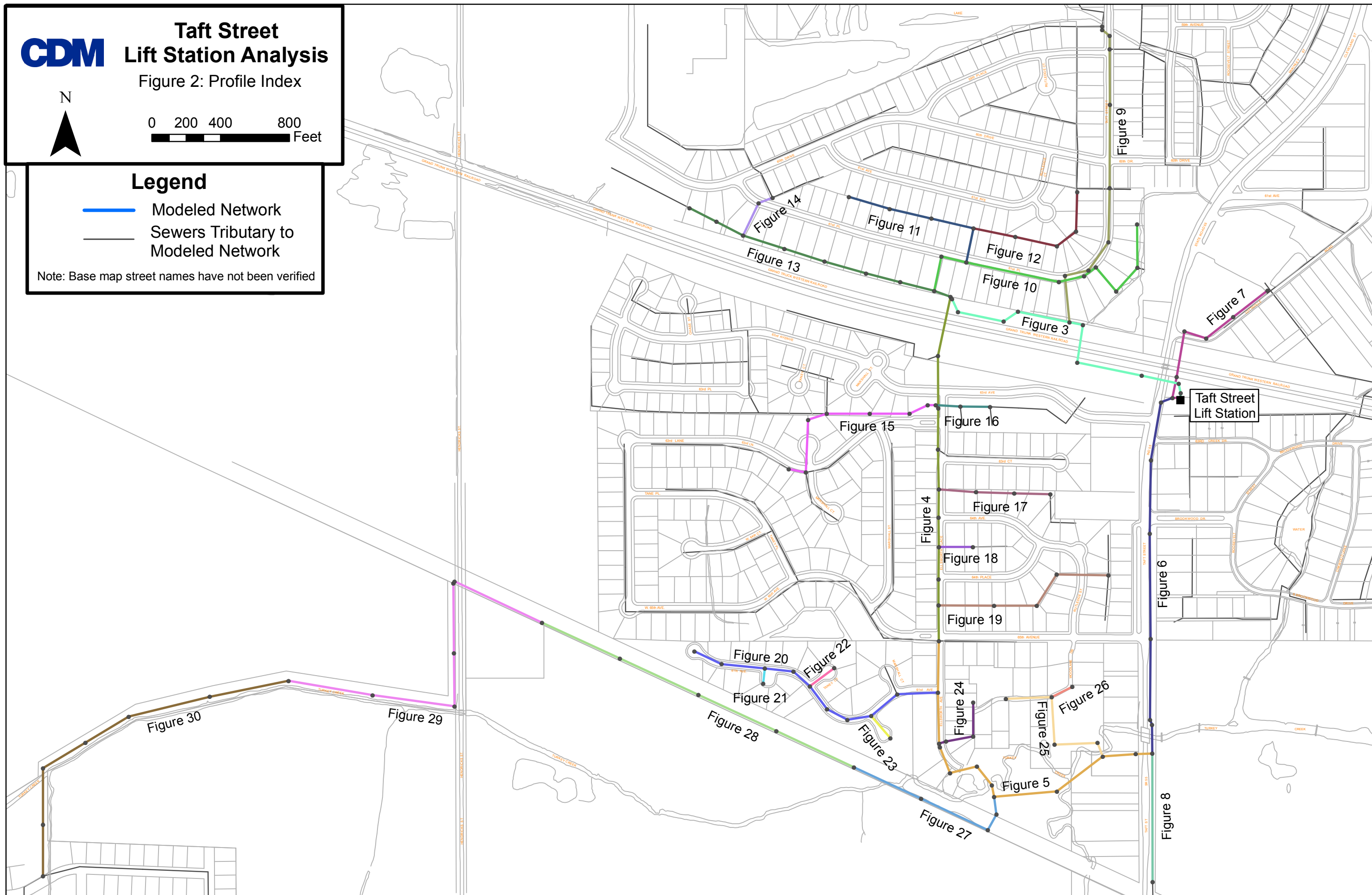


0 200 400 800 Feet

## Legend

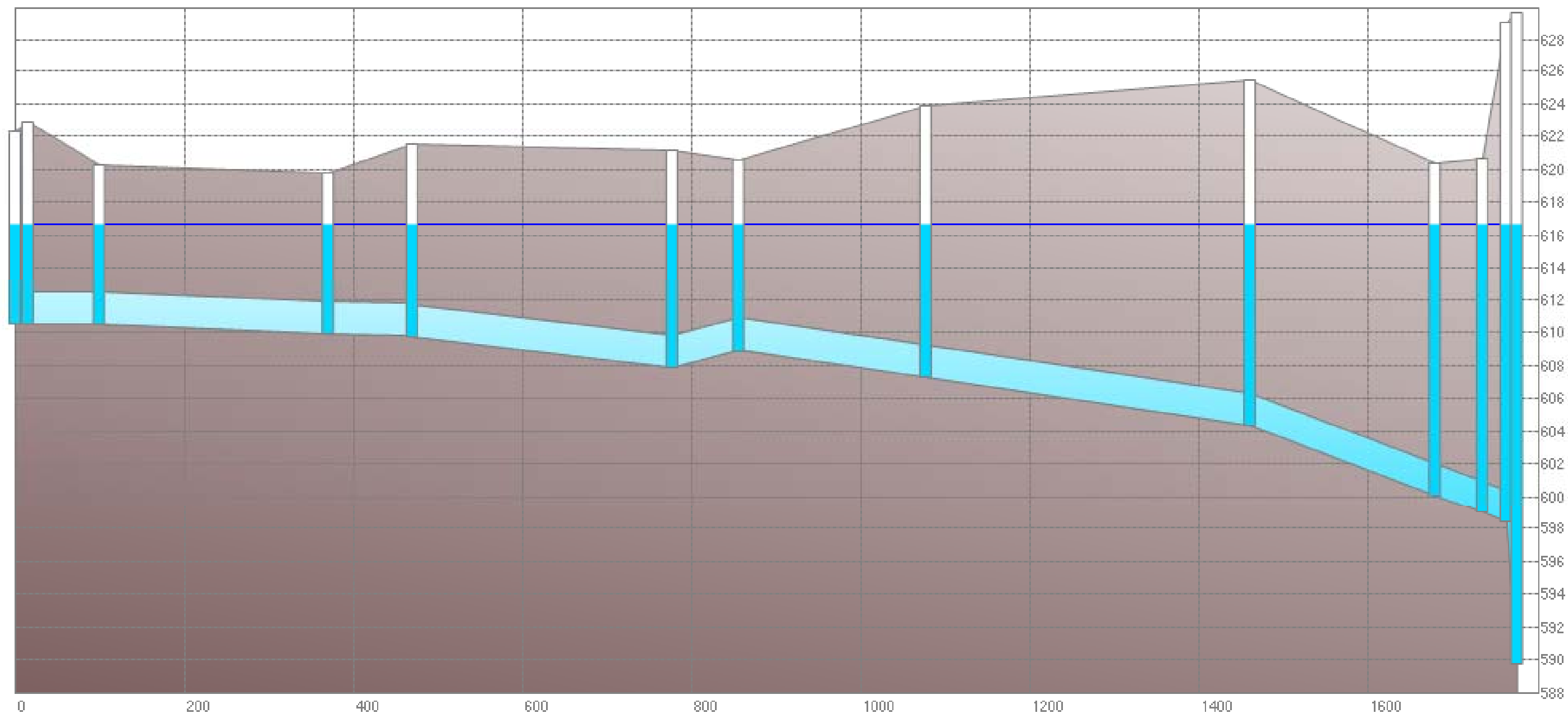
-  Modeled Network  
 Sewers Tributary to Modeled Network

Note: Base map street names have not been verified





— HGL

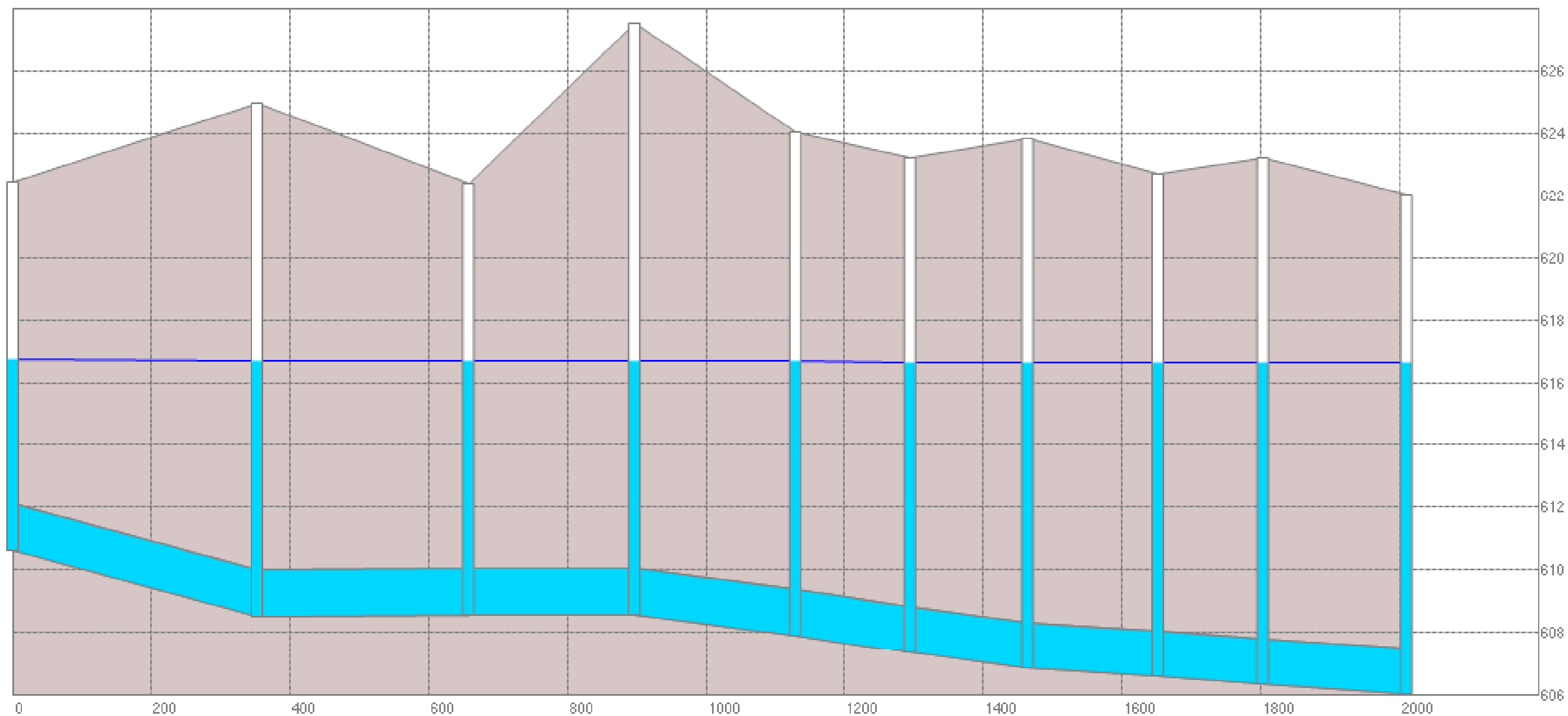


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 3  
Adjacent to Grand Truck Western  
Railroad to Lift Station (2-4' Dia.)



— HGL

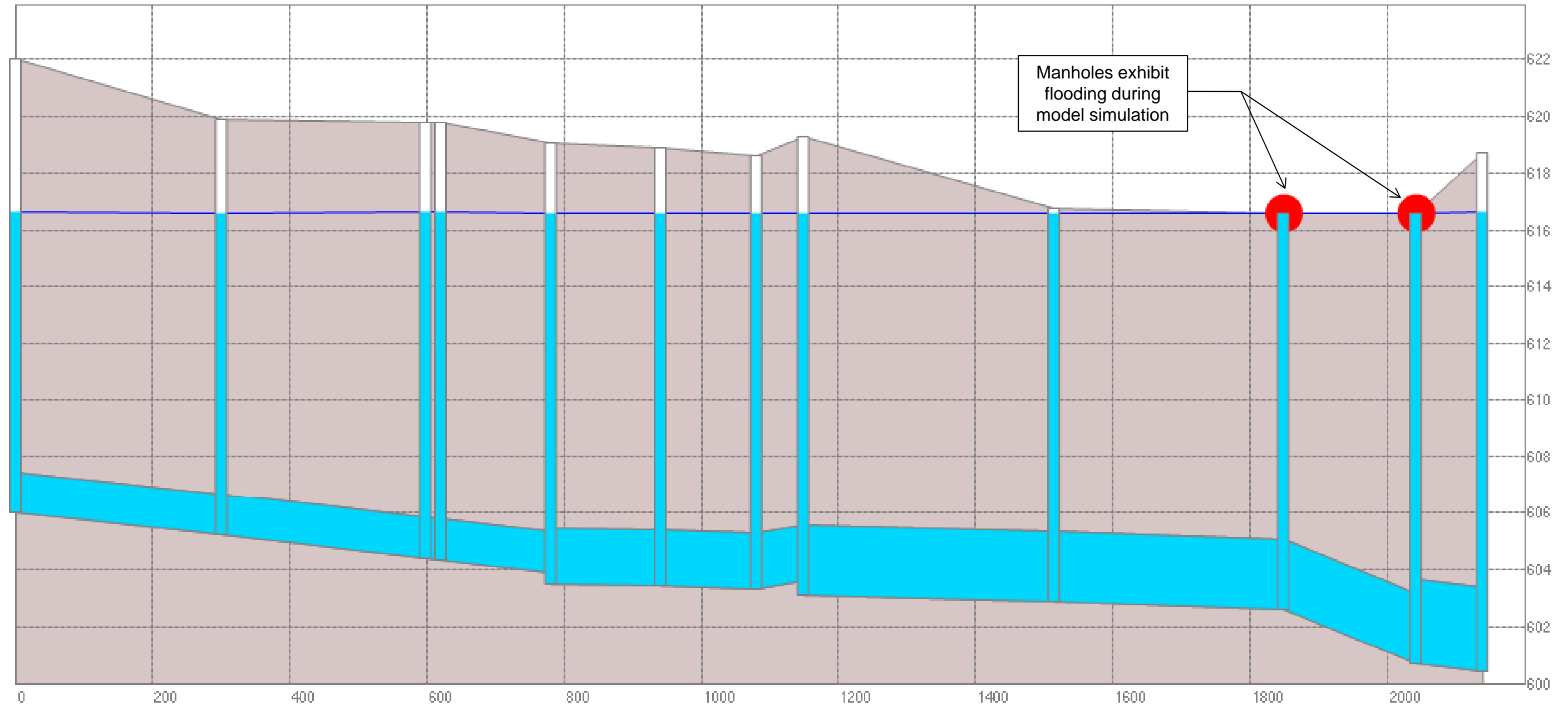


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 4  
Ellsworth Place from Railroad to 65<sup>th</sup>  
Avenue (1.5' Dia.)



— HGL

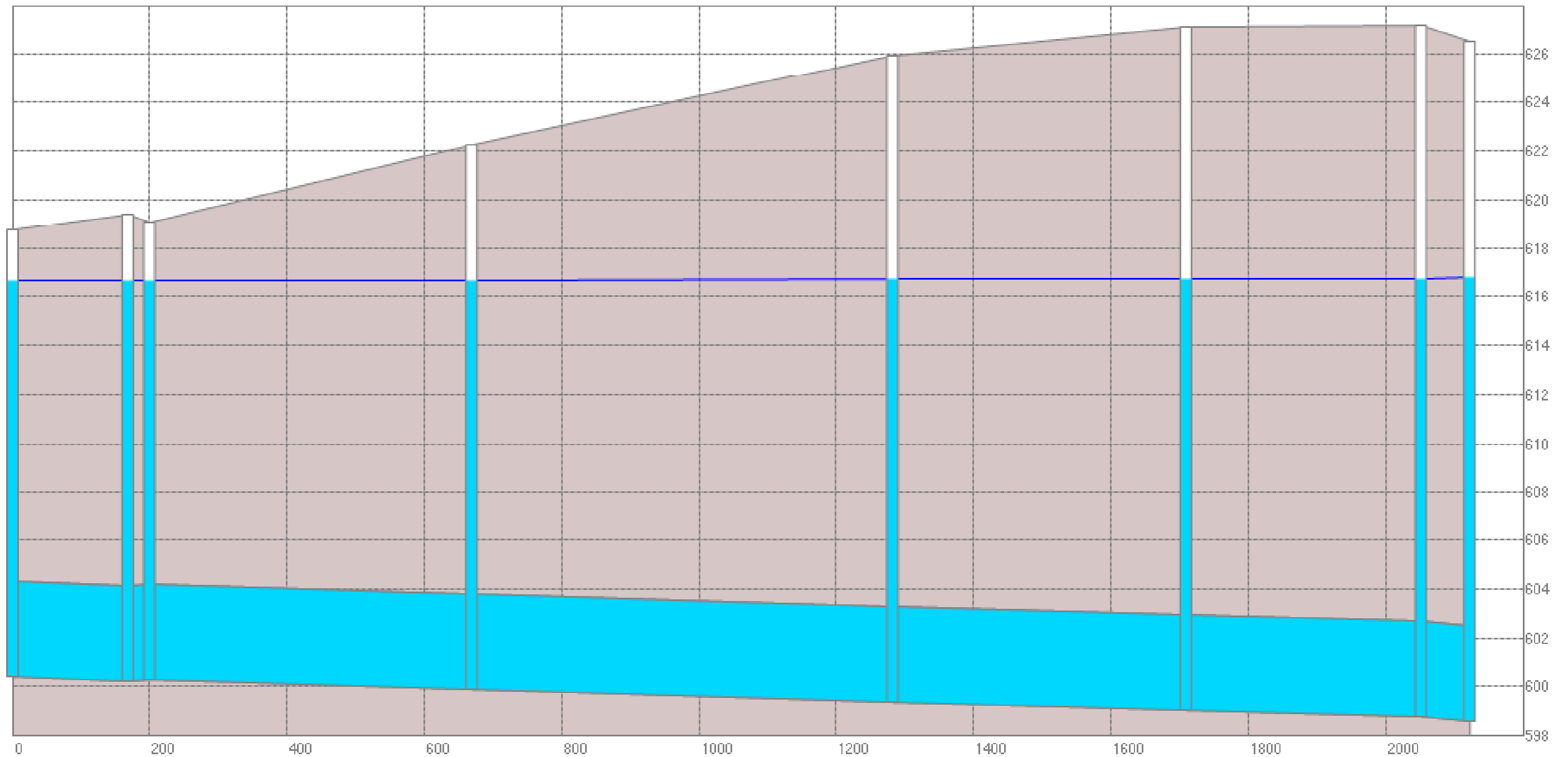


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 5  
Ellsworth Place from 65<sup>th</sup> Avenue to  
Taft Street (1.5-3' Dia.)



— HGL

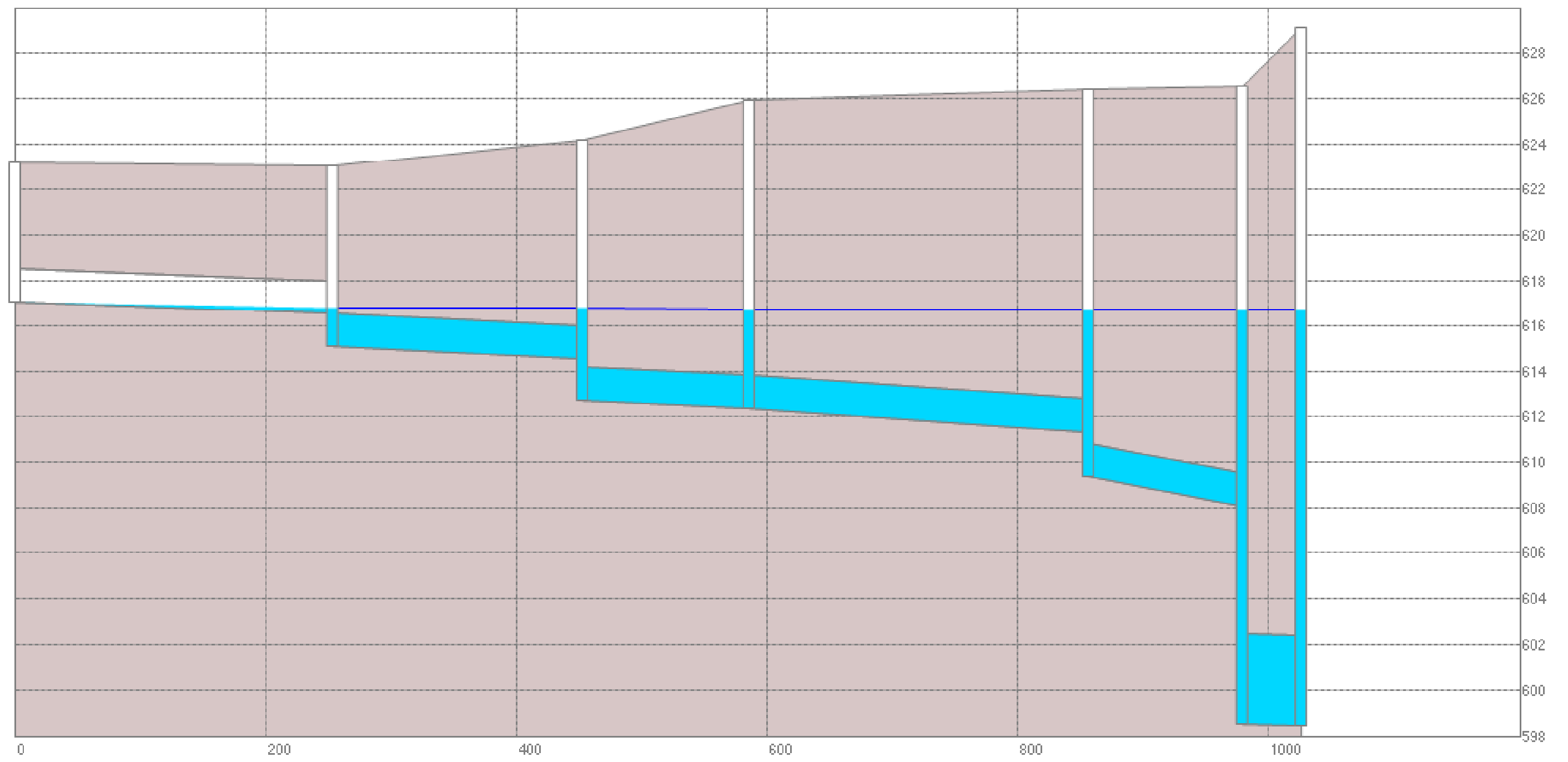


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 6  
Taft Street from Turkey Creek to Lift  
Station (4' Dia.)



— HGL

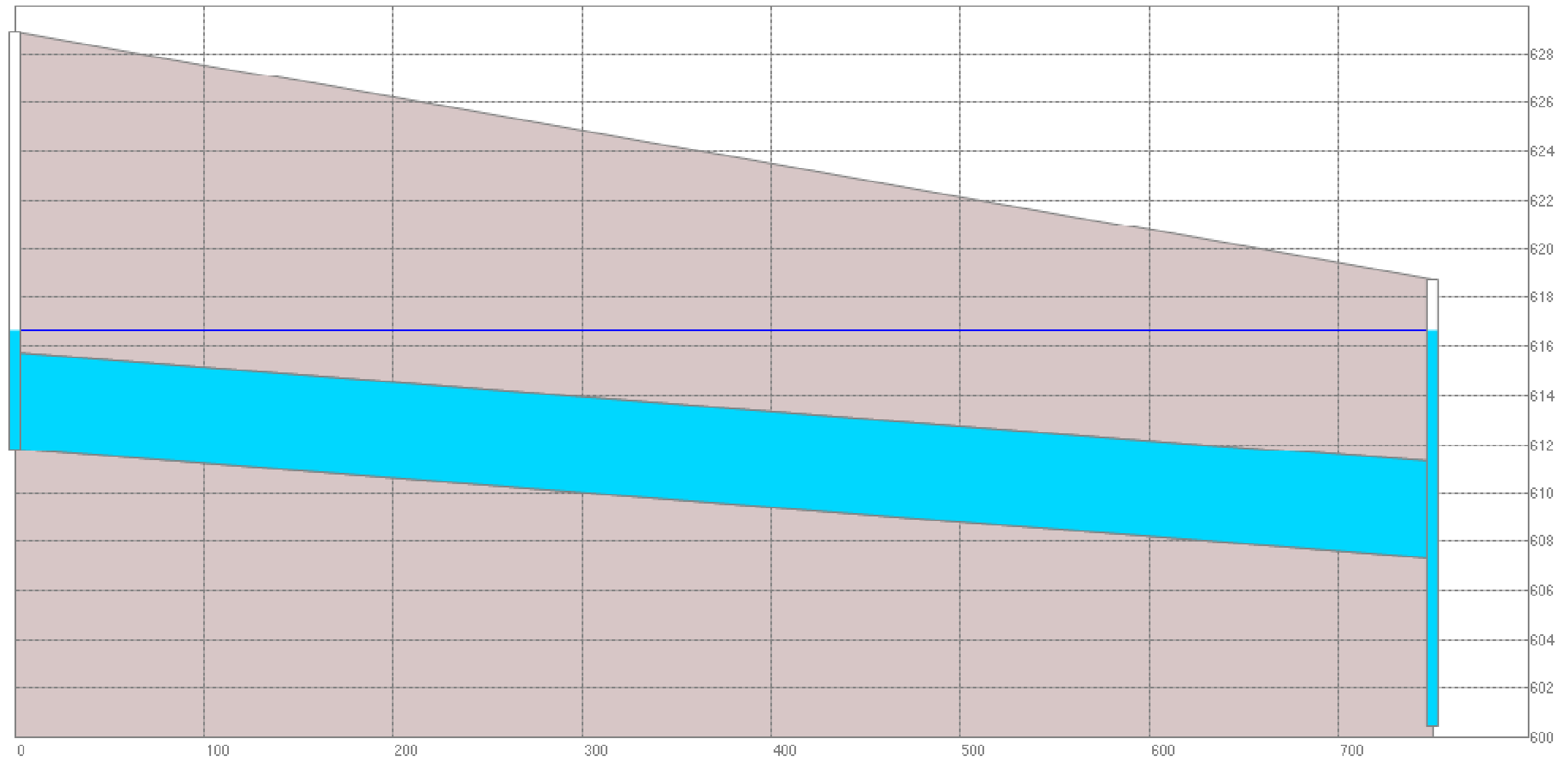


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 7  
Cleveland Circle to Lift Station  
(1.5'-4' Dia.)



— HGL

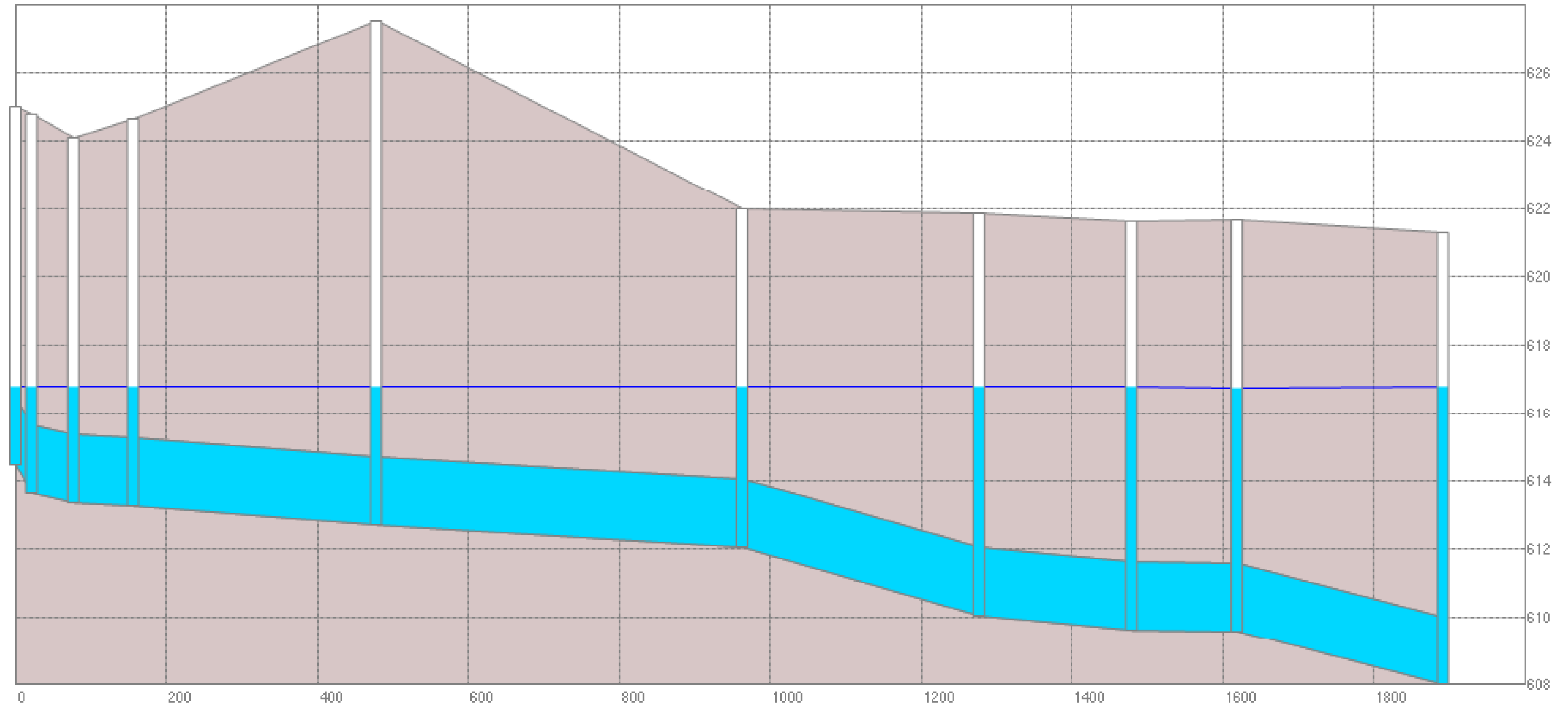


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 8  
Taft Street from Merrillville Conservancy  
District Boundary to Turkey Creek (4' Dia.)



— HGL

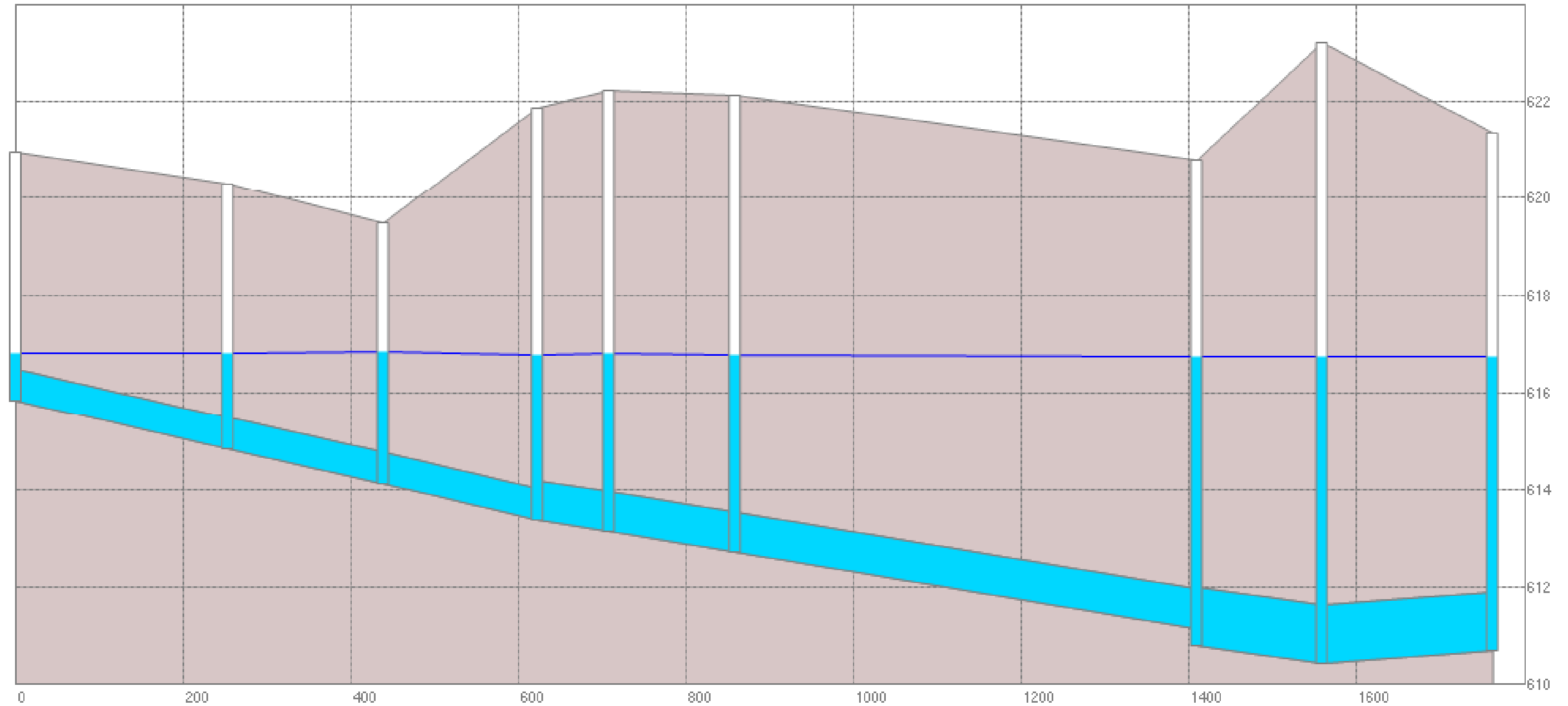


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 9  
Taft Place to Railroad  
(2' Dia.)



— HGL

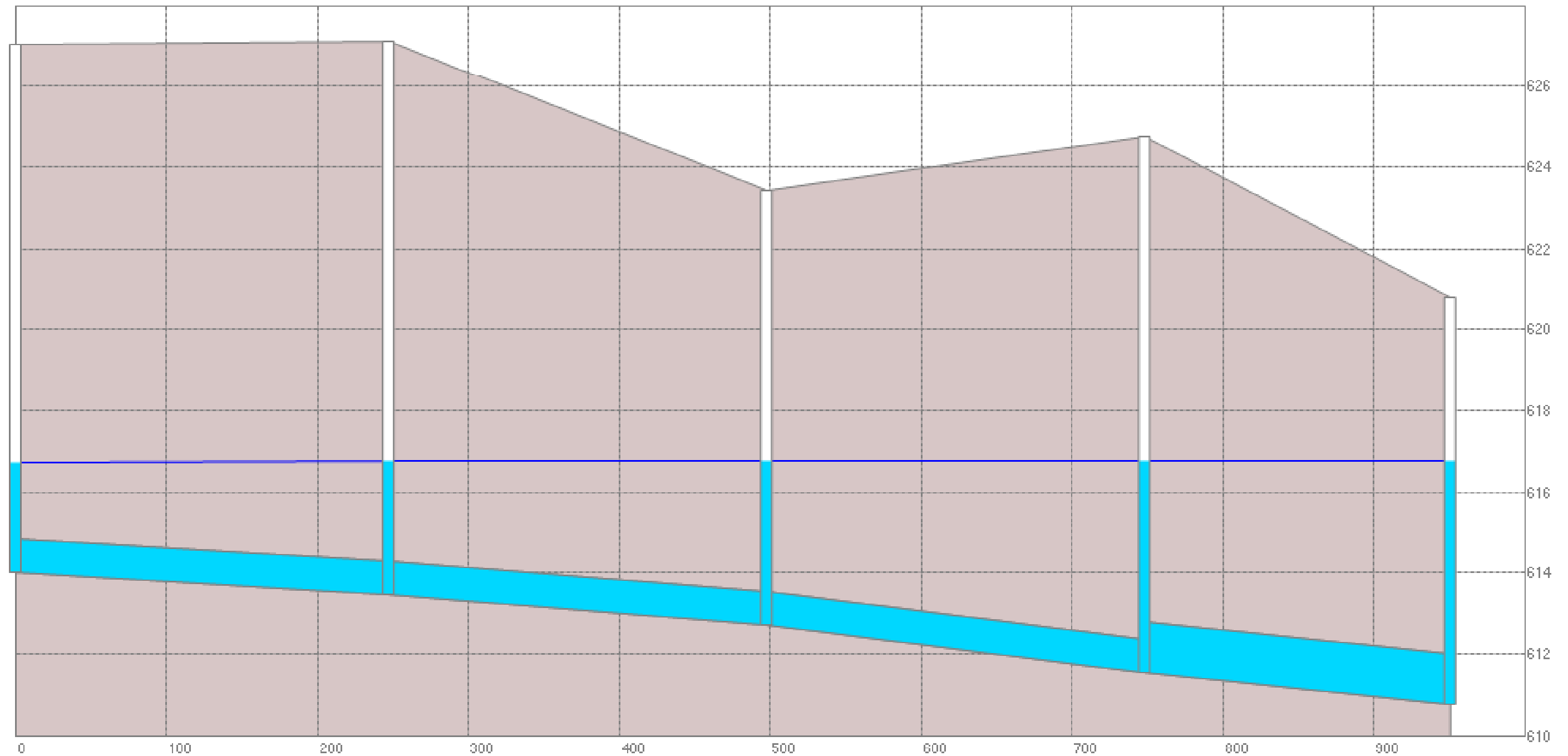


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 10  
Taft Place / 61<sup>st</sup> Place to Railroad  
(8"-1.25' Dia.)



— HGL

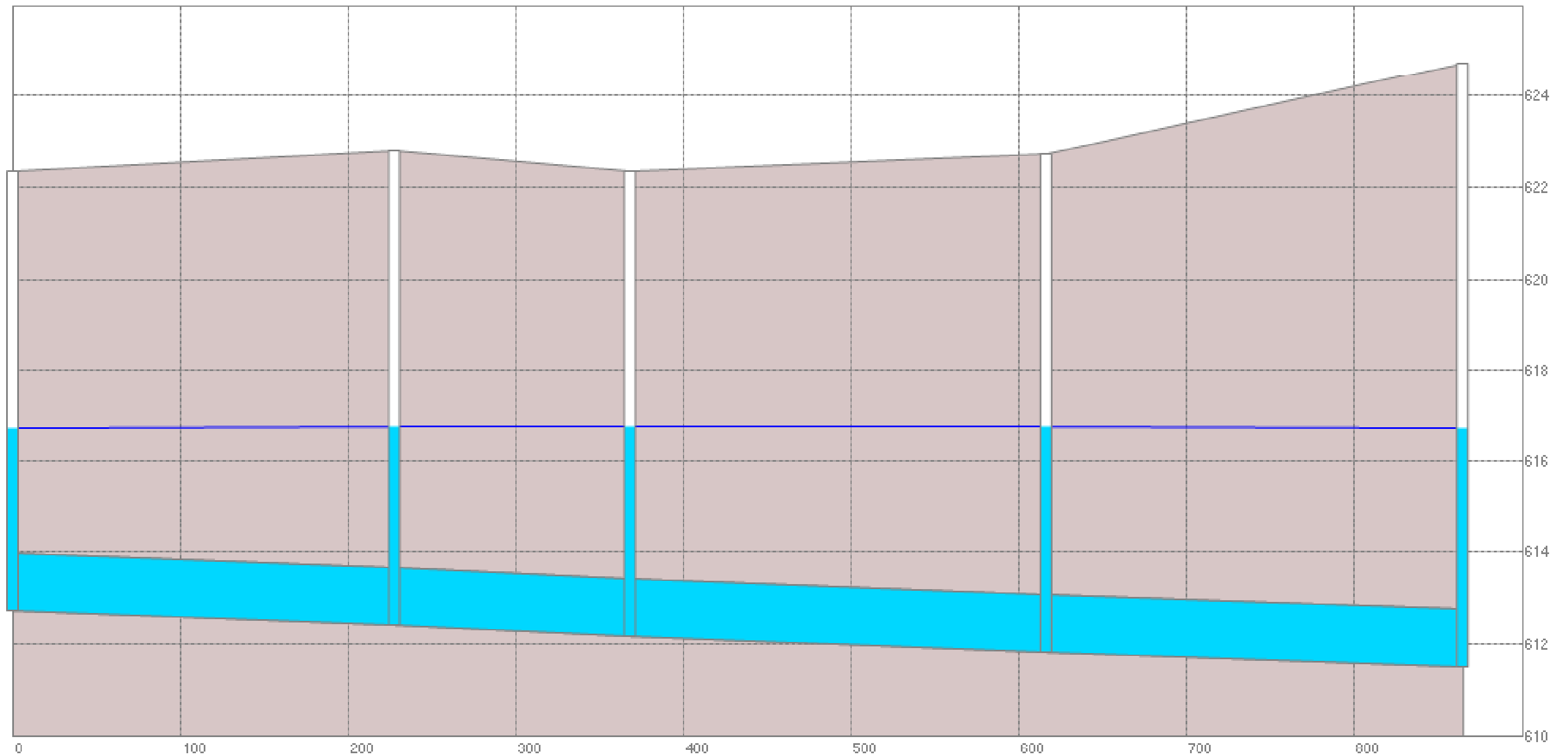


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 11  
Between 61<sup>st</sup> Avenue and 61<sup>st</sup> Place  
(10"-1.25' Dia.)



— HGL

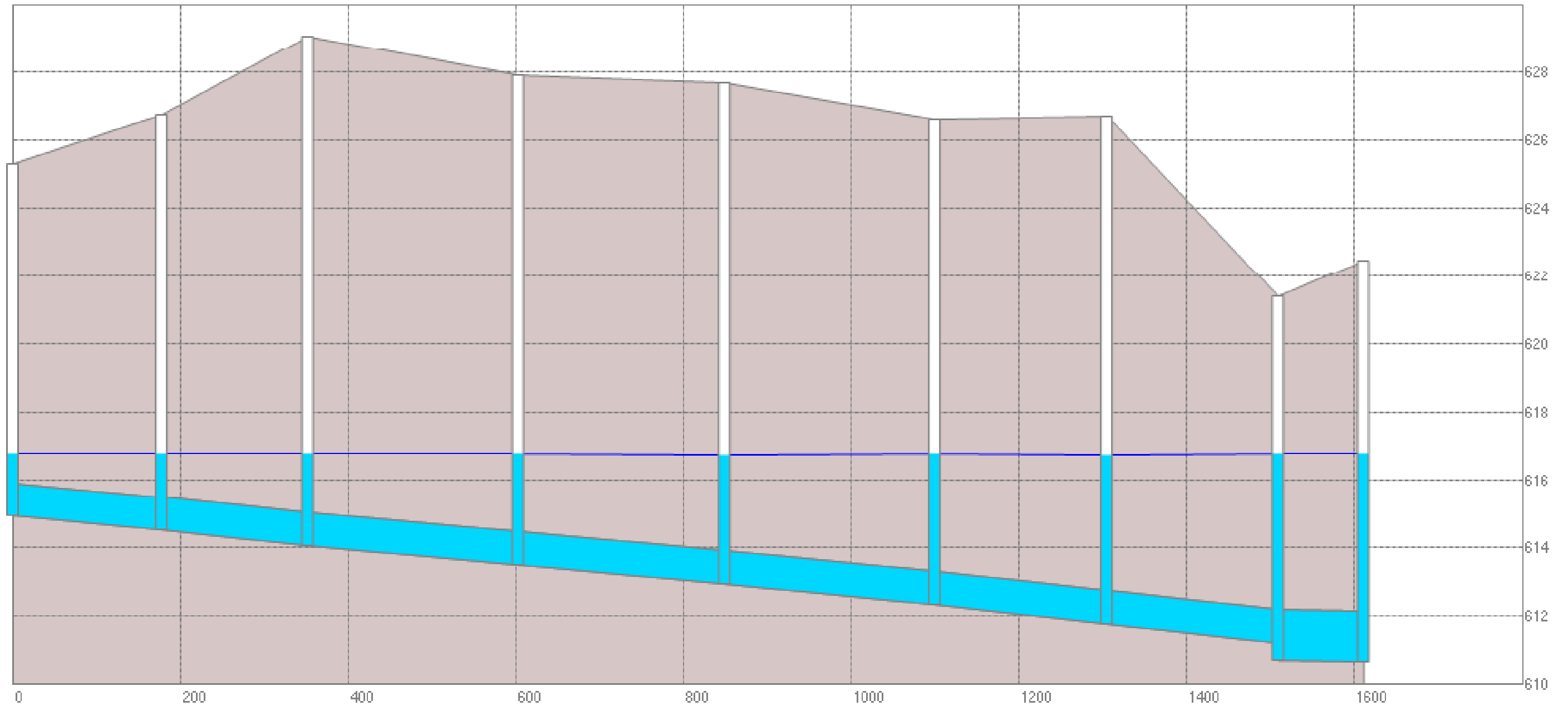


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 12  
South of 60<sup>th</sup> Drive Between Taft Place  
and Rutledge Court (1.25' Dia.)



— HGL

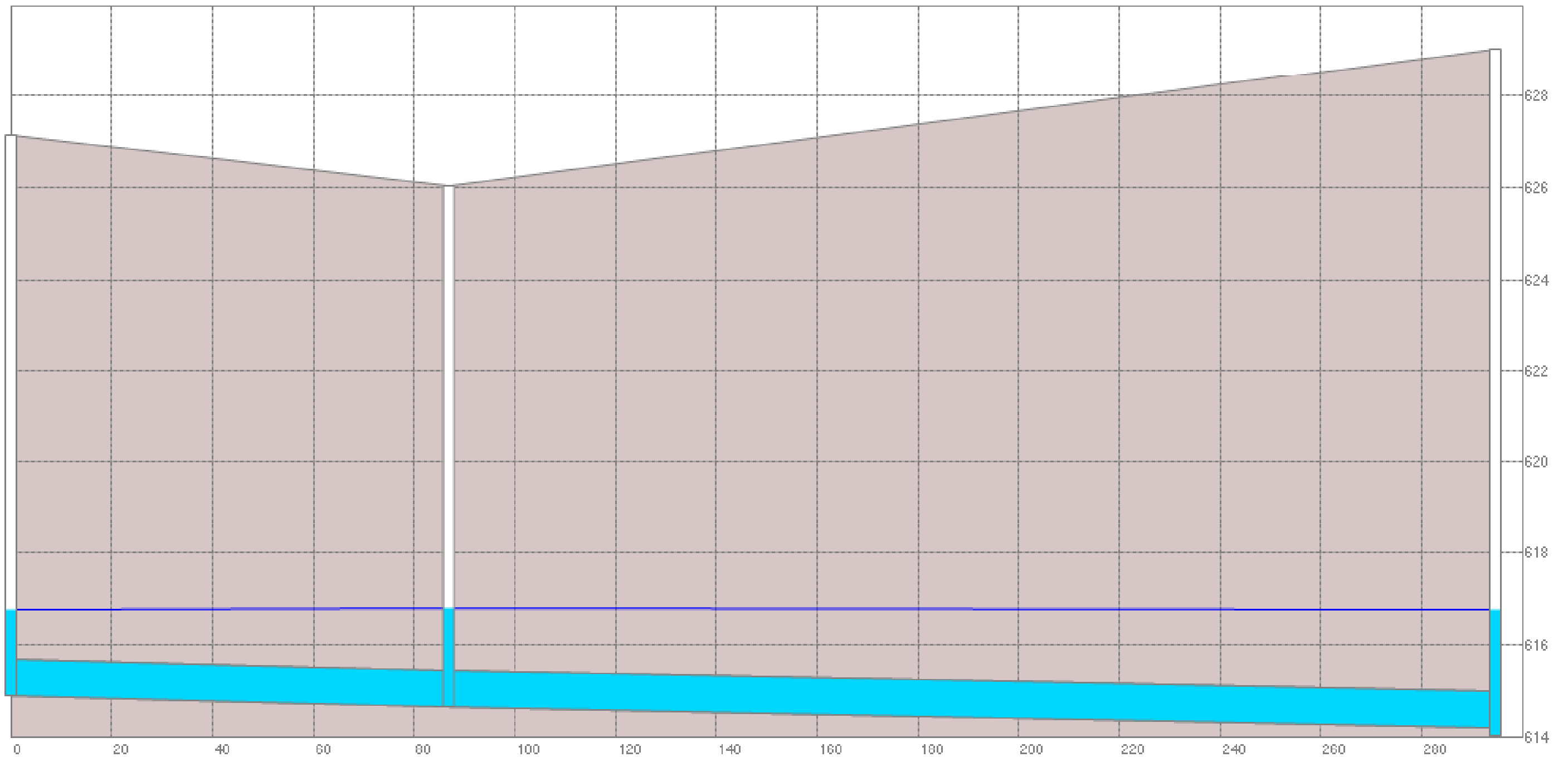


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 13  
Parallel to Grand Truck Western  
Railroad (1-1.5' Dia.)



— HGL

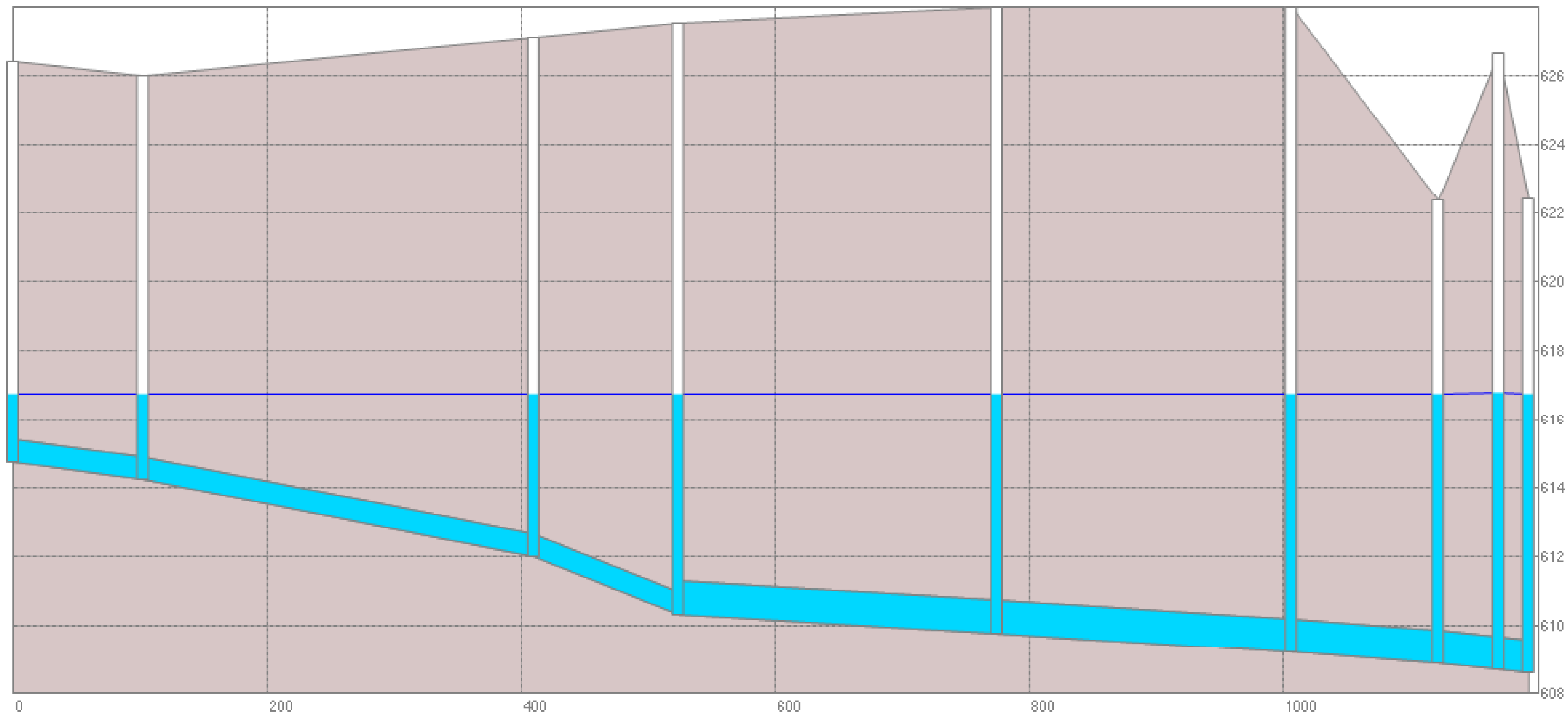


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 14  
East of Intersection of 60<sup>th</sup> Drive and  
61<sup>st</sup> Place to Railroad (10" Dia.)



— HGL

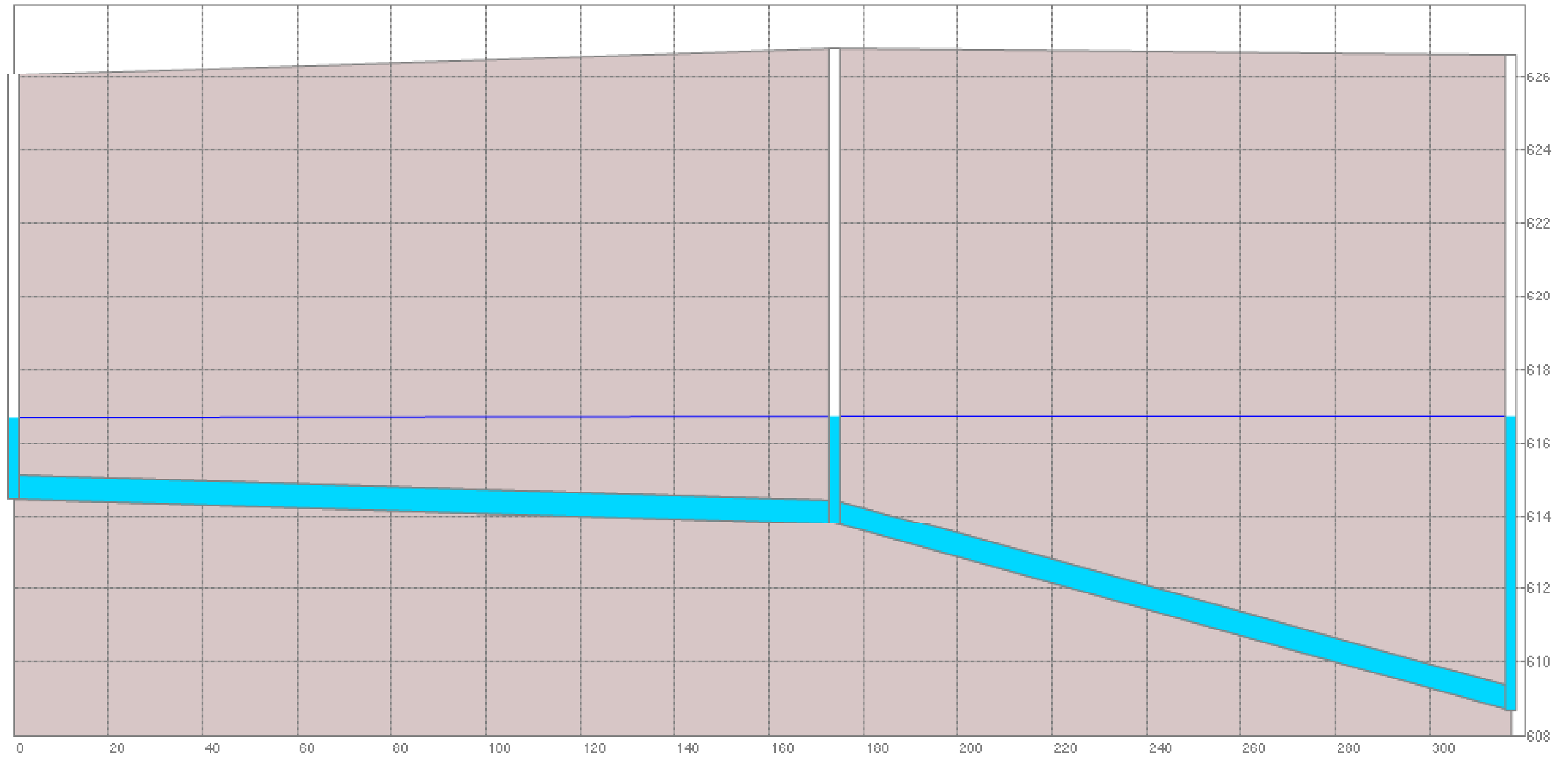


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 15  
63<sup>rd</sup> Lane from Marshall Court to  
Ellsworth Place (8"-1' Dia.)



— HGL

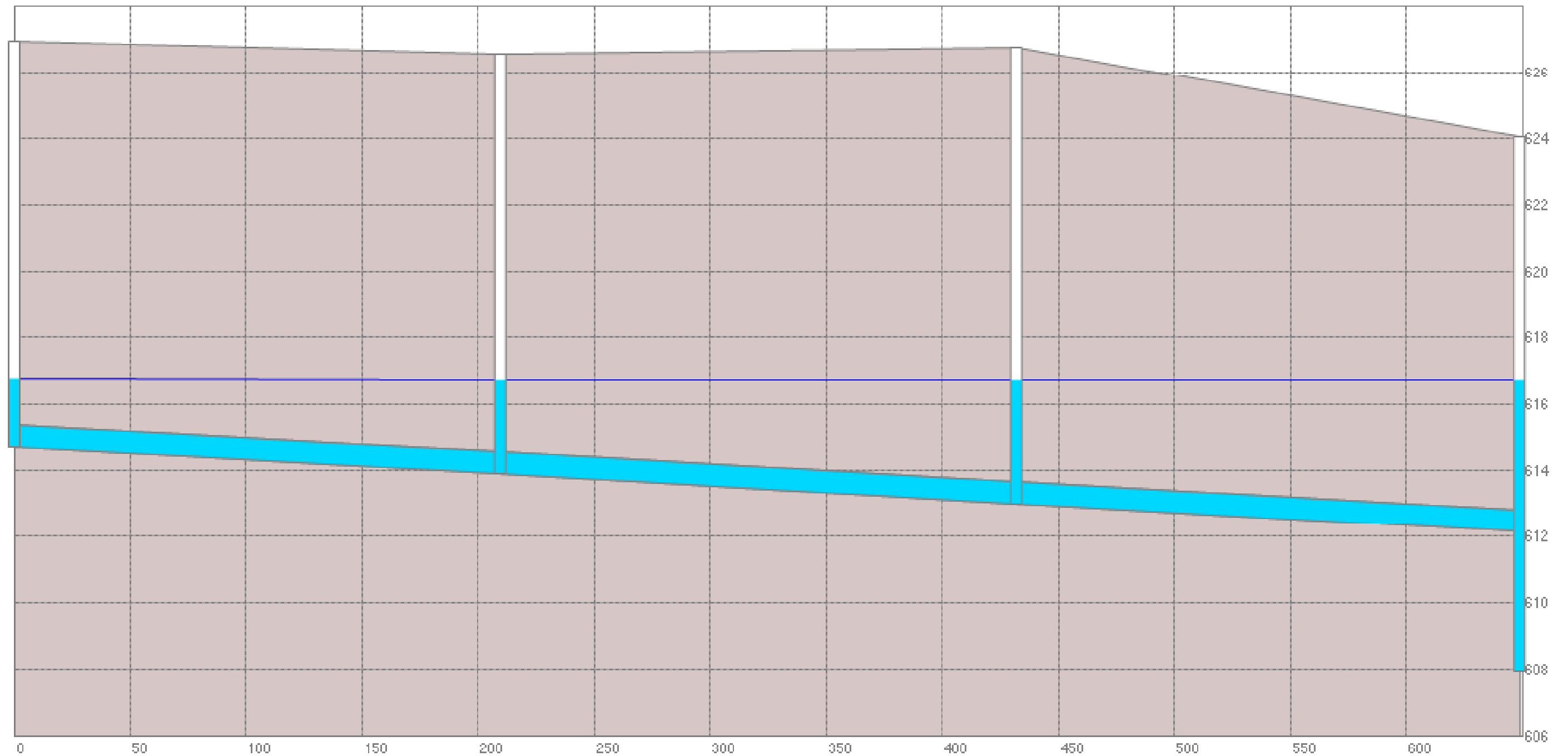


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 16  
East of Ellsworth Place, South of 63<sup>rd</sup>  
Avenue (8" Dia.)



— HGL

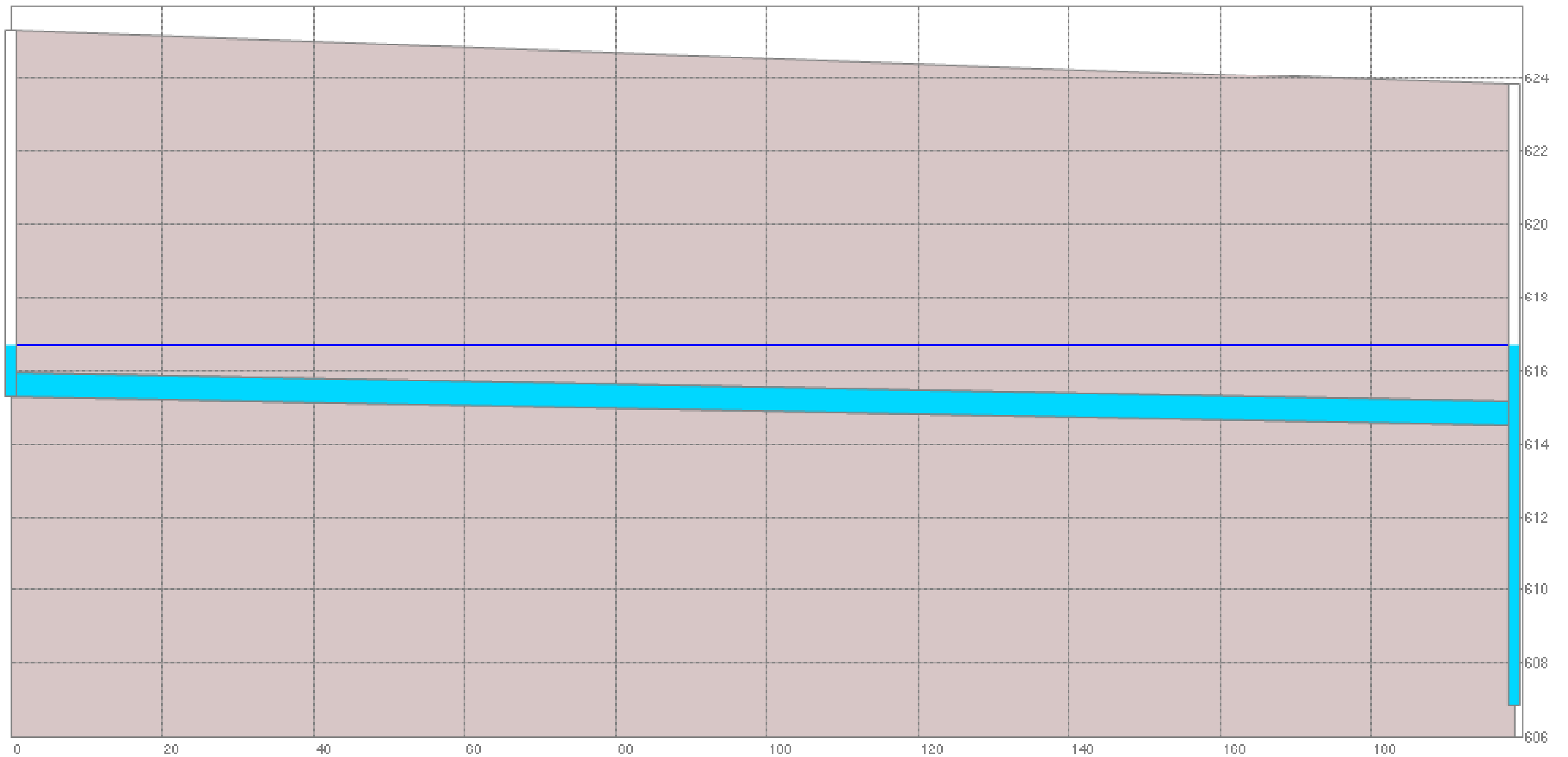


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 17  
East of Ellsworth Place Between 63<sup>rd</sup>  
Court and 64<sup>th</sup> Avenue (8" Dia.)



— HGL

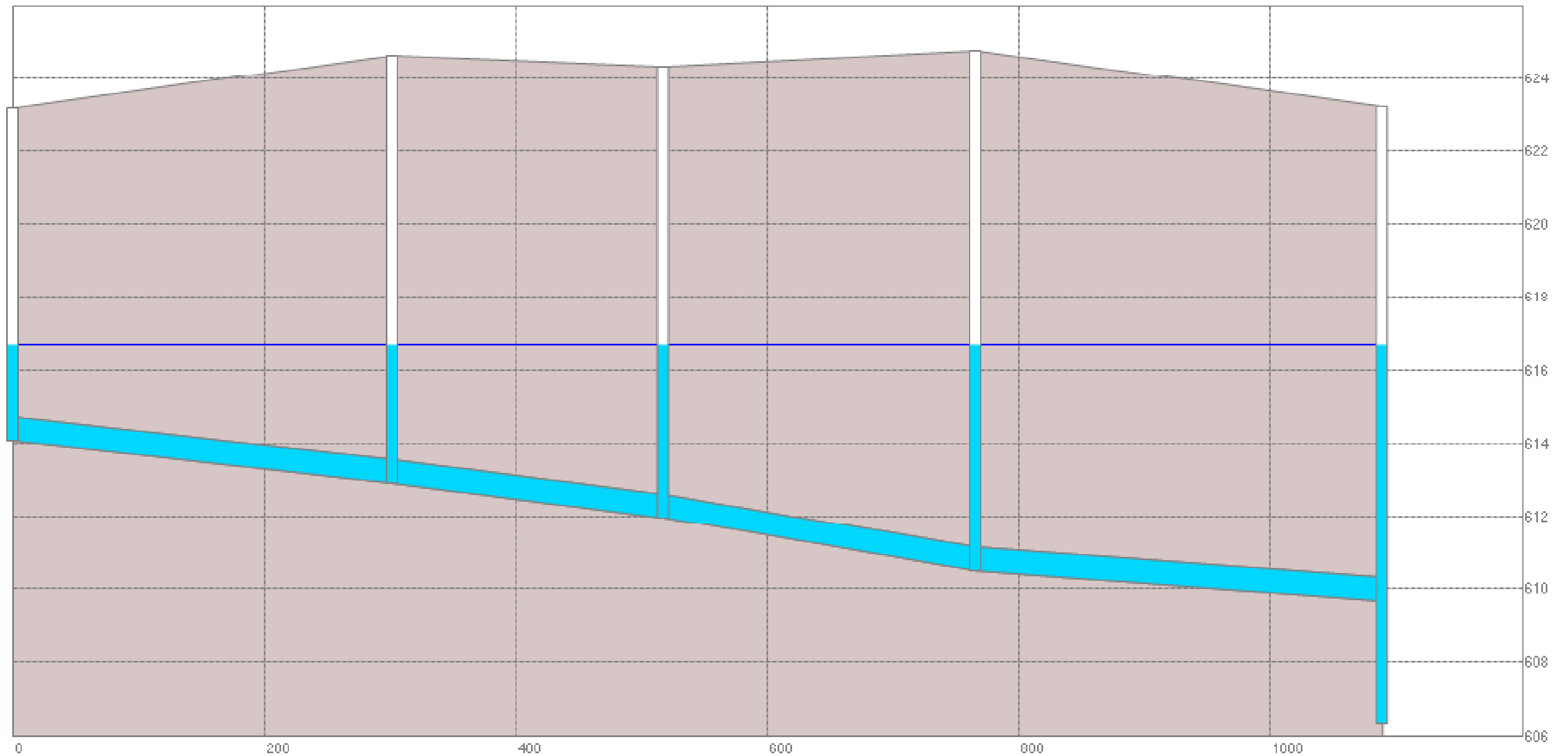


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 18  
East of Ellsworth Place Between 64<sup>th</sup>  
Avenue and 64<sup>th</sup> Place (8" Dia.)



— HGL

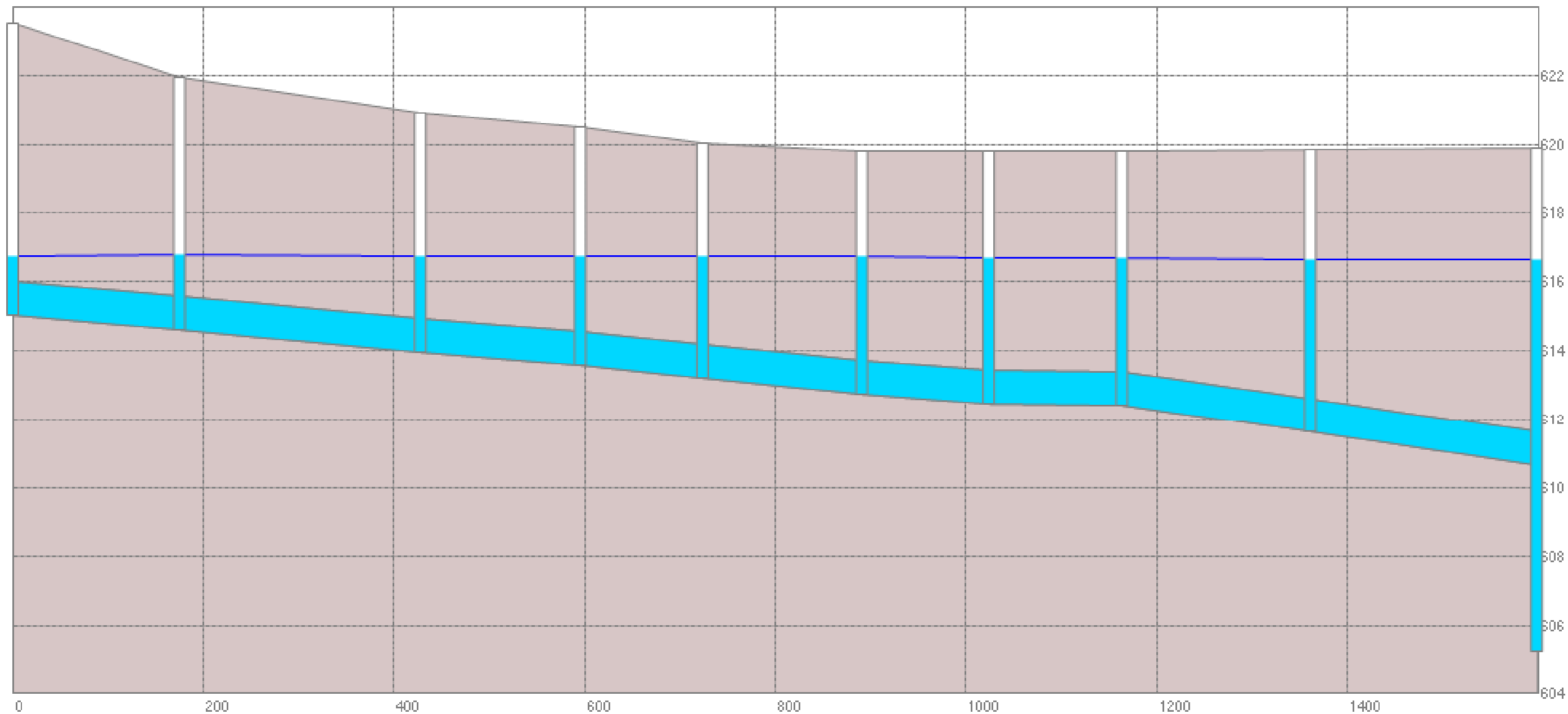


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 19  
East of Ellsworth Place Between 64<sup>th</sup>  
Place and 65<sup>th</sup> Avenue (8" Dia.)



— HGL

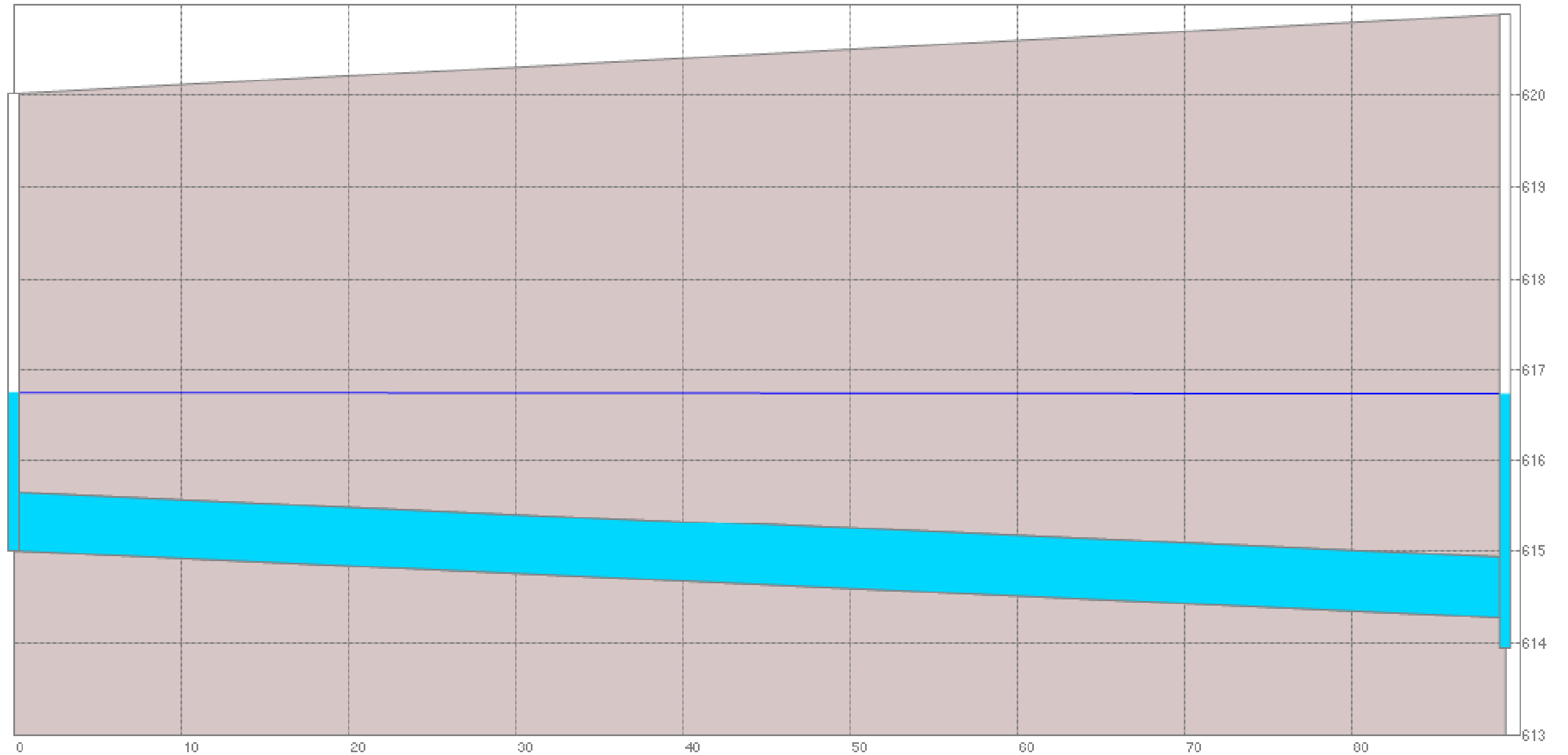


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 20  
65<sup>th</sup> Place  
(1' Dia.)



— HGL

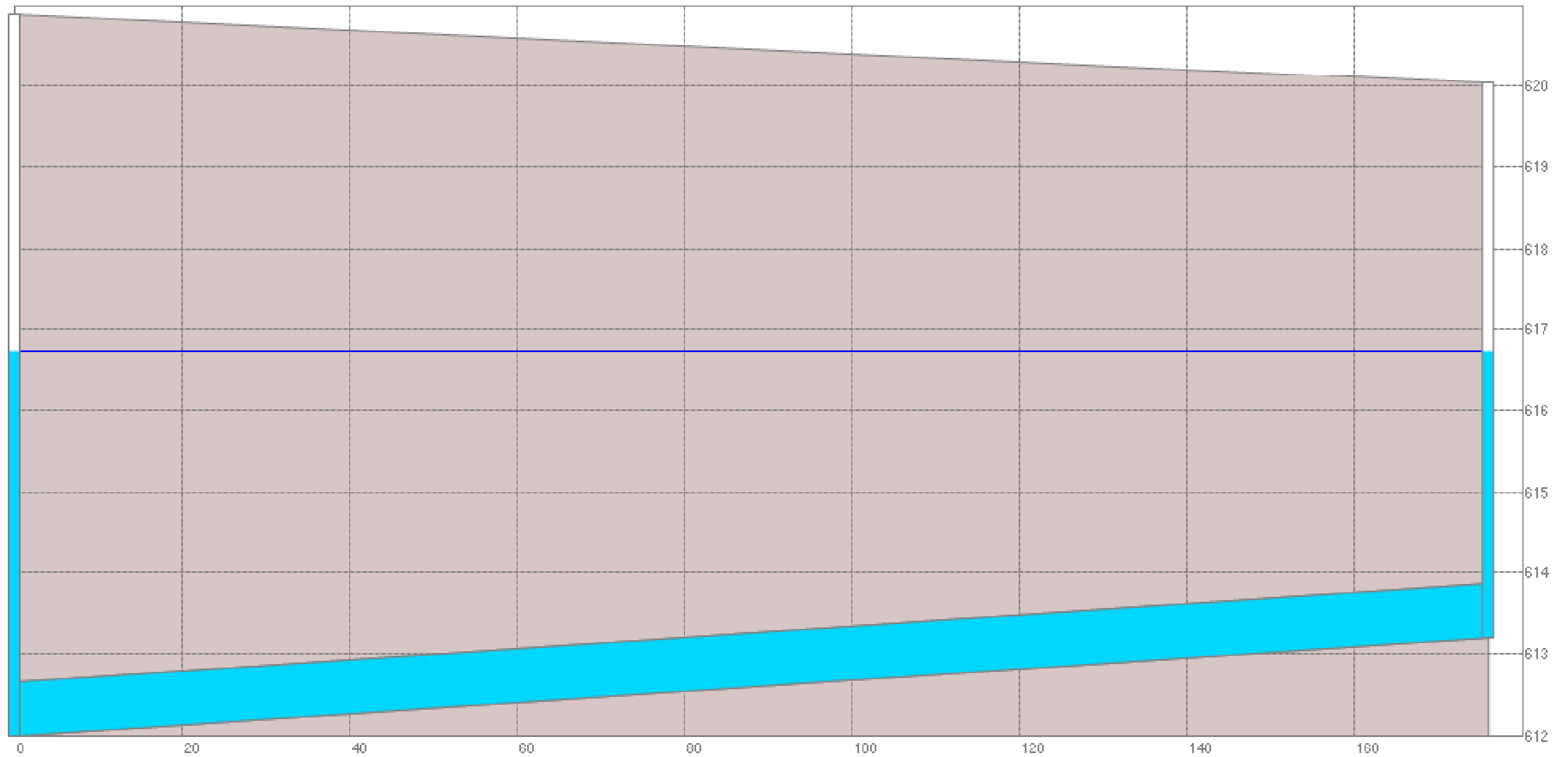


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 21  
Western Knuckle on 65<sup>th</sup> Place  
(8" Dia.)



— HGL

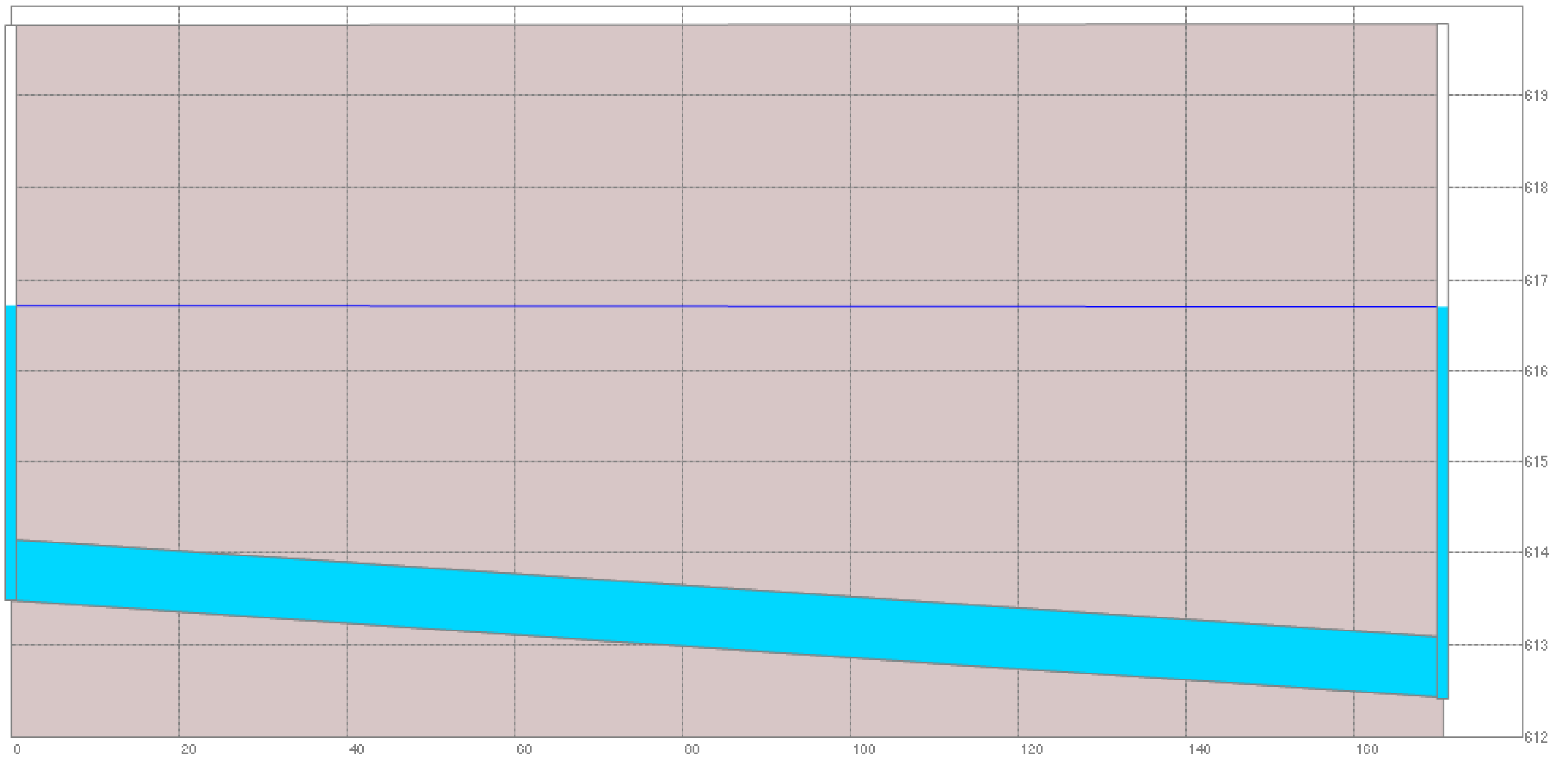


Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 22  
Taney Court to 65<sup>th</sup> Place  
(8" Dia.)



— HGL



Taft Street Lift Station Analysis  
Peak Hydraulic Grade Line for 3.2 MGD Constant Flow / Pump Failure Scenario

Figure 23  
Marshall Court to 65<sup>th</sup> Place  
(8" Dia.)